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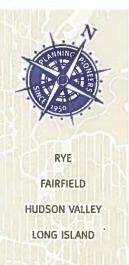
TRAFFIC ACCESS AND IMPACT STUDY

Proposed New LifeTime Fitness Building High Ridge Park Stamford, Connecticut



Prepared for: George Comfort & Sons, Inc. LifeTime

July 2017



FREDERICK P. CLARK ASSOCIATES, INC.

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Proposed New LifeTime Fitness Building High Ridge Park Stamford, Connecticut

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FREDERICK P. CLARK ASSOCIATES, INC.

PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT RYE, NEW YORK FAIRFIELD, CONNECTICUT

July 11, 2017

Ms. Megan Eaton
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2902 Corporate Place
Chanhassen, Minnesota 55317

Mr. Steven Ketchabaw Vice President George Comfort & Sons, Inc. 2 Manhattanville Road Purchase, New York 10577

Dear Ms. Eaton and Mr. Ketchabaw:

We are pleased to submit this Traffic Study, which was completed for the proposed LifeTime Fitness Building at High Ridge Park, located near Interchange 35 of the Merritt Parkway in Stamford, Connecticut. The proposed development comprises an 114,000 square-foot Fitness Center which will replace an existing 83,888 square-foot office building (Building 3) on-site and another 6,128 square-feet of office space located within the existing office park facility (which will be converted to storage). An access drive will be provided to the internal High Ridge Park access roadway.

It is anticipated that the proposed Fitness Center will generate a total of 161, 402 and 317 total trip ends, of which 56, 141 and 0 will be internal trips ends from the adjacent office buildings during the weekday morning, weekday afternoon and Saturday midday peak hours, respectively. If Building 3 were to remain and be reoccupied with medical uses (the most likely tenants based on current market trends) and the 6,128 square-foot office space (to be removed) remained, it is anticipated that it will generate a total of 210, 309 and 308 vehicle trip ends during the three peak hours, respectively. Therefore, the proposed development is anticipated to generate a net reduction of 105 and 48 vehicle trip ends during the weekday peak hours and a very modest increase of 9 total vehicle trip ends during the Saturday midday peak hour.

While results of the capacity and the storage/queue analyses indicate that the Study Area intersections generally operate with traffic delay during the Study Area peak hours, this condition is not exacerbated by the proposed development. Moreover, the construction of the proposed development will result in a net decrease in traffic impact when compared to the office use on the Campus. Further, details regarding these traffic conditions are included in the report.

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Ms. Megan Eaton Mr. Steven Ketchabaw Page 2 July 11, 2017

Based on the results of this traffic analysis, off-site traffic operational changes or roadway improvements are not needed to accommodate this new development and removal of an office use.

Sincerely,

Michael A. Galante

Executive Vice President

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Enclosure

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SUMMARY

A Traffic Access and Impact Study was prepared to provide the City of Stamford, Connecticut Department of Transportation (ConnDOT) and the Office of the State Traffic Administration (OSTA) with a detailed analysis to determine potential traffic impacts from the proposed LifeTime Fitness Building. The site is located in the westerly side within the High Ridge Park. The proposed 114,000 square-foot Fitness Center will replace the existing 83,888 square-foot Building 3 and approximately 6,128 square-feet of office space from the office park facility, which will be converted to storage space. For the purposes of completing the Study, the proposed development is expected to be completed and fully occupied by the end of 2019.

This Traffic Study addresses traffic conditions for the 2017 existing, the 2019 future no-build and build conditions during the weekday morning, weekday afternoon and Saturday midday peak hours of the adjacent street system. The 2017 baseline traffic volumes were extracted from manual turning movement counts conducted for all but one of the Study Area intersections in January and March of 2017 during the weekday morning, weekday afternoon and Saturday midday peak periods. Traffic volumes for the intersection of High Ridge Road at Cedar Heights Road/Turn of River Road were derived from another Traffic Study prepared by this office.

Future no-build traffic volumes, without the proposed development, assumed that the currently vacant Building 3 was re-occupied with medical uses (because current market trends indicate that this would be the most likely user group). A 0.6 percent annual growth rate was employed to the existing traffic volumes to the design year of 2019 as required by ConnDOT. No other developments were identified by either the City of Stamford Planning Department or the ConnDOT Planning Division for inclusion in this analysis. The annual growth rate is consistent with the City of Stamford and ConnDOT rates.

Traffic generation for both the vacant Building 3 re-occupancy with medical uses and the proposed LifeTime Fitness building are based on trip generation rates provided by the Institute of

Transportation Engineers (ITE). It is anticipated that the removal of re-occupied Building 3 with medical office and the removal of 6,128 square-feet of office space from the existing office park facility will generate a total of 210, 309 and 308 fewer vehicle trip ends during the weekday morning, weekday afternoon and Saturday midday peak hours, respectively.

It is anticipated that the proposed Fitness Center will generate a total of 161, 402 and 317 total trip ends, of which 56, 141 and 0 are internal trips and a 105, 261 and 317 are external vehicle trip ends during the weekday morning, weekday afternoon and Saturday midday peak hours, respectively. The net difference between the re-occupancy of Building 3 with medical uses and the proposed development is a reduction of 105 and 48 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively, and an increase of 9 vehicle trip ends during the Saturday midday peak hour. Thus, the proposed development will have a reduced traffic impact on area roadways during weekday peak hours compared to the no build condition and only generate 9 additional trips during the Saturday midday peak hour.

Based on a review of current traffic patterns at Study Area intersections, anticipated travel routes to the site and guidance from ConnDOT, a distribution plan was developed. As described above, the site access drives will be to the internal High Ridge Park driveway. It is assumed that all traffic will use the northerly internal access drive, to be conservative. It was determined that 20 percent of the site traffic will arrive from and depart to the east and west (each way), respectively, on State Route 15, 25 percent arrive from and depart to the east on Intervale Road, 15 percent will arrive from and depart to the north on High Ridge Road, 13 percent will arrive from and depart to the south on High Ridge Road and 7 percent will arrive from and depart to the west on Cedar Heights Road.

Future 2019 build traffic volumes were developed based on adding the net increase in site traffic generation to the 2019 no-build traffic volumes, as previously described.

A SYNCHRO 9 macroscopic capacity analysis was conducted for the 2017 existing, 2019 nobuild and build conditions to identify incremental traffic impacts and needs that the proposed development will generate during peak hours.

Results of the capacity analysis and the storage/queue analysis indicate that certain Study Area intersections will continue to operate with traffic congestion during the Study Area peak hours. However, this condition is not exacerbated by the proposed development. In fact, in some instances, conditions will improve, and the construction of the proposed development will not result in a net increase in traffic impact during the weekday peak hours when compared to the permitted office use in the Campus.

INTRODUCTION

This report has been prepared for submission to the City of Stamford, ConnDOT and OSTA to identify potential impacts and need for mitigation, if any, for the proposed LifeTime Fitness building, which will replace an existing office building. It will be located within High Ridge Park.

This report addresses typical weekday morning, weekday afternoon and Saturday midday peak hours to determine potential impact to adjacent and nearby roadways. The Study includes current roadway conditions, existing traffic volumes, accident experience, site traffic generation, assignment of this traffic and future build traffic volumes.

Project Description

The proposal is to construct an 114,000 square-foot Fitness Center, which will replace the existing 83,888 square-foot Building 3 coupled with the removal of 6,128 square-foot office space (converted to storage space) from the existing office park facility. Access will be from within High Ridge Park. For purposes of completing this Traffic Study, it is assumed the design year is the end of 2019.

EXISTING CONDITIONS

This section of the Traffic Report includes a description of the roadway inventory, nearby intersections, the results of recent traffic counts, traffic control and accident experience.

Roadways

As noted above the site is located on the westerly side of High Ridge Park. The following is a description of nearby roadways:

- High Ridge Road This is a north-south, generally four-lane, State-maintained roadway, also designated State Route 137. It begins at the signalized intersection with Bedford Street/Summer Street/Long Ridge Road to the south of the Study Area and continues north. Traffic signals and turning lanes are provided at key intersections. To the north of the Study Area, it continues as a two-lane roadway passing into New York State. The State Route 15 interchange 35 ramps are controlled with traffic signals. It provides a double yellow centerline, shoulder lines and curbing in the Study Area. Certain sections of this road have sidewalks. A raised median is provided between Dunn Avenue and Buxton Farm Road. The roadway width is generally 58 feet to the south and 44 feet to the north of Buxton Farm Road, respectively. It has a posted speed limit is 40 miles per hour. CT Transit Bus Route 331 provides service along this road. Land use in the Study Area is mostly commercial development.
- 2. <u>Buxton Farm Road</u> This is an east-west, two-lane, city-maintained roadway. It begins at the signalized intersection with High Ridge Road and continues in the easterly direction and terminates at the All-Way STOP controlled intersection with Turn of River Road. It provides a double yellow centerline and curbing. A sidewalk and NO PARKING ANYTIME sign is provided along the southerly side of the road from the Shopping Center middle driveway to the intersection with Turn of River Road. CT Transit Bus Route 331B provides stops along this road. Land uses are commercial.

- 3. <u>Turn of River Road</u> This is mainly a north-south, two-lane, city-maintained roadway. It begins at the end of the High Ridge Park driveway and continues south terminating at the signalized intersection with High Ridge Road. Generally this road has the right-of-way, with most side roads being controlled with STOP signs. The intersections with Buxton Farm Road and Intervale Road are All-Way STOP controlled. It provides a double yellow centerline and curbing. This roadway has posted speed limit of 25 miles per hour in the Study Area. Land use is mainly residential.
- 4. <u>Intervale Road</u> This is an east-west, two-lane, city-maintained roadway. It begins at the All-Way STOP controlled intersection with Turn of River Road and continues east. Just beyond the intersection with Joan Road this roadway continues as Newfield Drive. On this road the posted speed limit is 25 miles per hour. It provides a double yellow centerline, shoulder lines and curbing. A sidewalk is provided along the southerly side of the road. Land use is residential.
- 5. <u>Cedar Heights Road</u> This is an east-west, two-lane, city-maintained roadway. It begins at the All-Way STOP controlled intersection with Wire Mill Road and continues east terminating at the signalized intersection with High Ridge Road. On this road the posted speed limit is 30 miles per hour. It provides a double yellow centerline, shoulder lines, curbing and sidewalks. Land use is residential.
- 6. <u>High Ridge Park</u> This is a north-south and east-west, two-lane, private access roadway. It begins as a continuation of Turn of River Road and enters High Ridge Park serving the 6 office buildings. A center median and curbing are provided. This roadway has a posted speed limit of 20 miles per hour. CT Transit Bus Route 331B provides stops along this road.
- 7. <u>Merritt Parkway</u> Although the Merritt Parkway is designated a north-south, State-maintained roadway, in Fairfield County it generally runs in an east-west direction. It is also designated State Route 15. It is a four-lane, median-divided parkway, limited to mostly passenger

vehicles. There is a full-movement interchange located at Interchange 35, which provides access to High Ridge Road, also designated State Route 137.

Figure 1 provides a graphic illustration of the site's location. Figure 2 provides a graphic illustration of the current street system characteristics. Photographs and the CT Transit Bus Route 331 schedule are included in the Appendix.

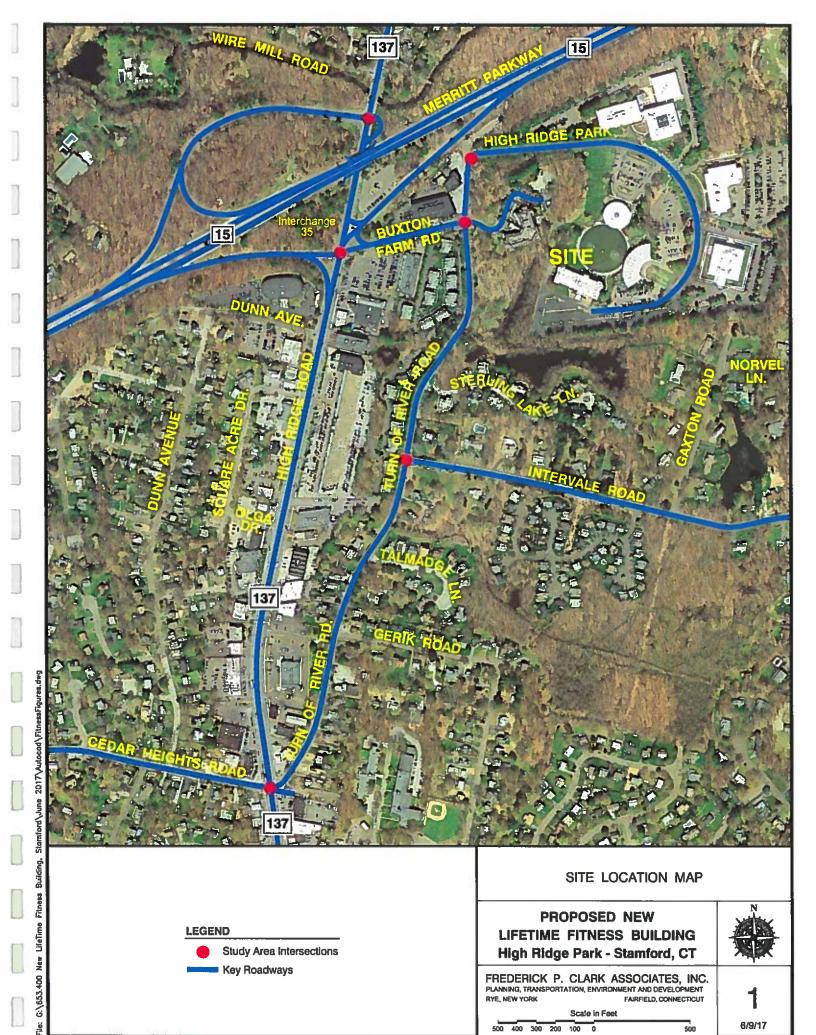
Traffic Volumes

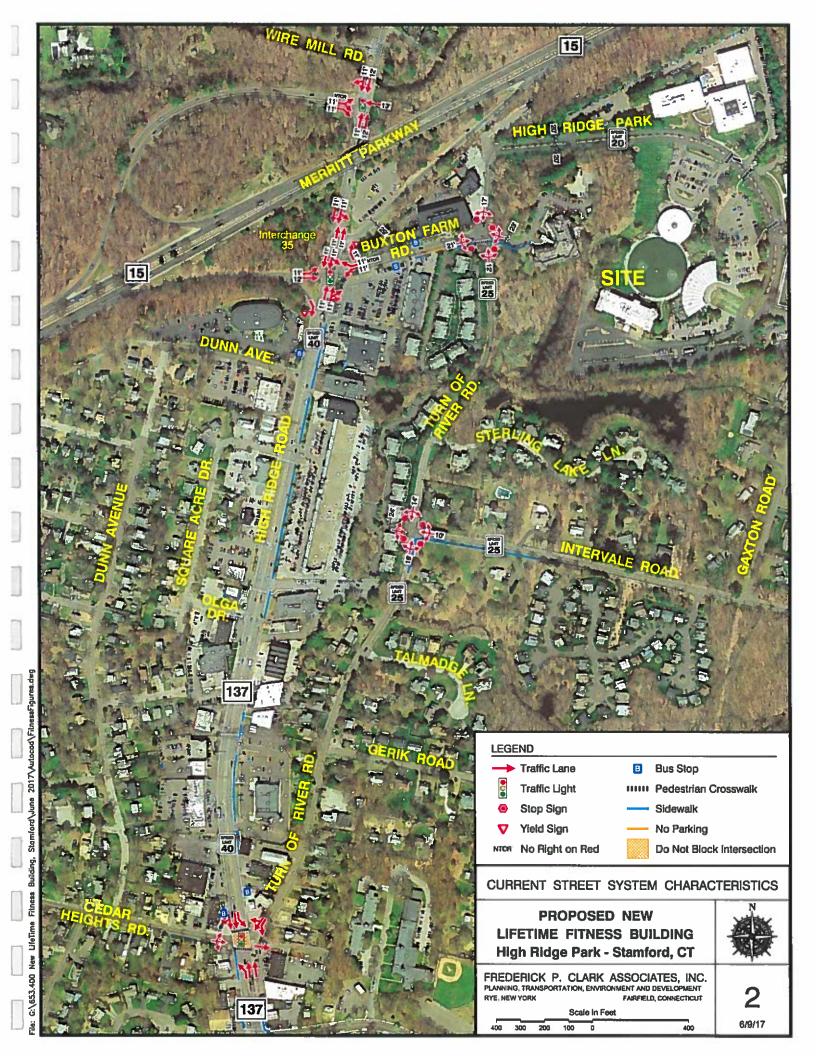
To complete this Traffic Study, it was necessary to collect baseline traffic volumes at key intersections near the subject property during the weekday morning, weekday afternoon and Saturday midday peak hours. The following intersections were included in the Study Area:

- High Ridge Road (State Route 137) at Merritt Parkway (State Route 15) Southbound Ramps (Signalized);
- High Ridge Road (State Route 137) at Merritt Parkway (State Route 15) Northbound Ramps/Buxton Farm Road (Signalized);
- Turn of River Road at Intervale Road/Access Drive;
- Turn of River Road at Buxton Farm Road/Assisted Living Access Drive;
- Turn of River Road at High Ridge Park; and,
- High Ridge Road (State Route 137) at Turn of River Road and Cedar Heights Road (Signalized).

Field surveys were conducted at the three Study Area unsignalized intersections along Turn of River Road on Thursday, January 26, 2017 and at the Interchange 35 intersections on Thursday, March 9, 2017 during the following time periods:

- Weekday morning 7:00 to 9:00 A.M.; and,
- Weekday afternoon 4:00 to 6:00 P.M.





For the Saturday midday condition, field surveys were conducted at the all of the Study Area intersections, except the High Ridge Road (State Route 137) at Turn of River Road and Cedar Heights Road intersection, on Saturday, January 28, 2017 during the following time period:

Saturday Midday – 10:00 A.M. to 2:00 P.M.

Traffic volumes for the intersection of High Ridge Road (State Route 137) at Turn of River Road and Cedar Heights Road were derived from a Traffic Study prepared by this office for all three time periods. The traffic volumes were obtained for the weekday morning, weekday afternoon and Saturday midday peak hours in November 2016. Based on the results of the field surveys the following peak hour volumes were identified:

- Weekday morning 7:45 to 8:45 A.M.;
- Weekday afternoon 5:00 to 6:00 P.M.; and,
- Saturday Midday 12:45 to 1:45 P.M.

Merritt Parkway Southbound Ramps, west of High Ridge Road, had a two-way volume of 1,840, 1,217 and 838 vehicles during the weekday morning, weekday afternoon and Saturday peak hours, respectively. North of the High Ridge Road/Merritt Parkway Southbound Ramps, High Ridge Road had a recorded two-way volume of 2,364, 2,042 and 1,371 vehicles during the weekday morning, weekday afternoon and Saturday peak hours, respectively. South of the High Ridge Road/Merritt Parkway Southbound Ramps, this roadway had a recorded two-way volume of 3,118, 2,777 and 1,809 vehicles during the weekday morning, weekday afternoon and Saturday peak hours, respectively.

West of High Ridge Road/Merritt Parkway Northbound Off-Ramp had a one-way volume of 684, 694 and 391 vehicles during the same three peak hours noted above. High Ridge Road/Merritt Parkway Northbound On-Ramp had a one-way volume of 765, 986 and 597 vehicles during the weekday morning and weekday afternoon peak hours, respectively. East of High Ridge Road, Buxton

Farm Road had a recorded two-way volume of 1,339, 1,328 and 691 vehicles during the same three peak hours noted above. South of the Merritt Parkway Northbound Ramps/Buxton Farm Road, High Ridge Road had a recorded two-way volume of 2,866, 2,887 and 2,028 vehicles during the same three peak hours noted above.

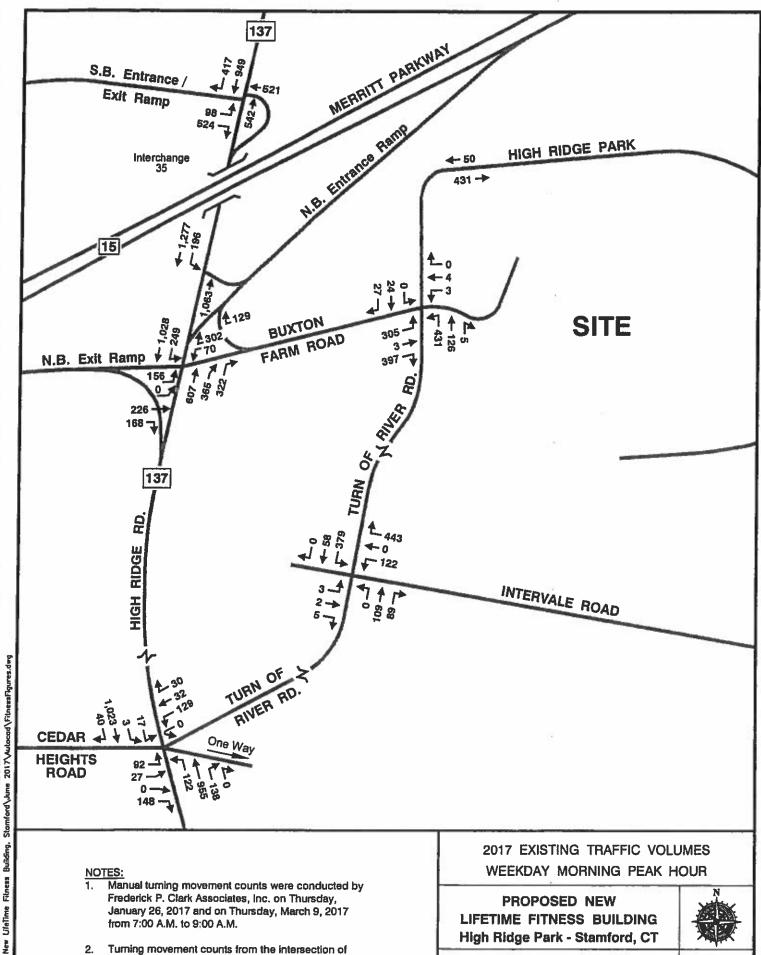
Turn of River Road, south of Buxton Farm Road/Assisted Living Access Drive, had a two-way volume of 986, 895 and 467 vehicles during the weekday morning, weekday afternoon and Saturday peak hours, respectively. North of Buxton Farm Road/Assisted Living Access Drive, Turn of River Road had a two-way volume of 482, 536 and 24 vehicles during the weekday morning, weekday afternoon and Saturday peak hours, respectively. Buxton Farm Road, west of Turn of River Road had a two-way volume of 1,167, 1,101 and 475 vehicles during the same three peak hours noted above.

North of Intervale Road/Access Drive, Turn of River Road had a two-way volume of 992, 877 and 463 vehicles during the weekday morning, weekday afternoon and Saturday peak hours, respectively. South of Intervale Road/Access Drive, Turn of River Road had a two-way volume of 383, 380 and 192 vehicles during the weekday morning, weekday afternoon and Saturday peak hours, respectively. Intervale Road, east of Turn of River Road had a two-way volume of 1,044, 848 and 519 vehicles during the same three peak hours noted above.

Figures 3 through 5 graphically illustrate the peak hour volumes, by turning movements, for the weekday morning, weekday afternoon and Saturday peak hours, respectively. Table 1 provides a more detailed summary of the recorded traffic volumes on area roadways near the subject property.

Accident Experience

The latest available accident data was obtained from ConnDOT for a period beginning September 1, 2013 through December 31, 2014 and from the UCONN Crash Data Repository (a new format and process) for a period beginning January 1, 2015 through August 31, 2016 for State Route 137, Buxton Farm Road and Turn of River Road. For the intersection of High Ridge Road the Merritt Parkway Northbound Off-Ramp Right Turn there were a total of two accidents recorded during this



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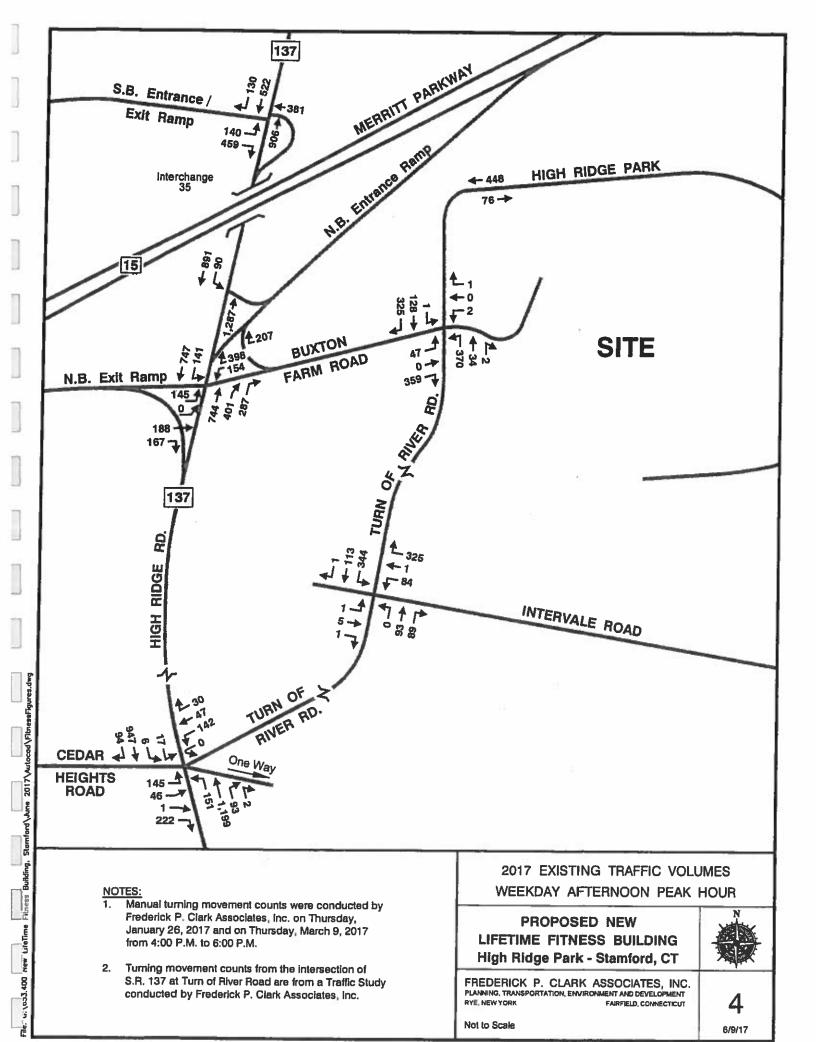
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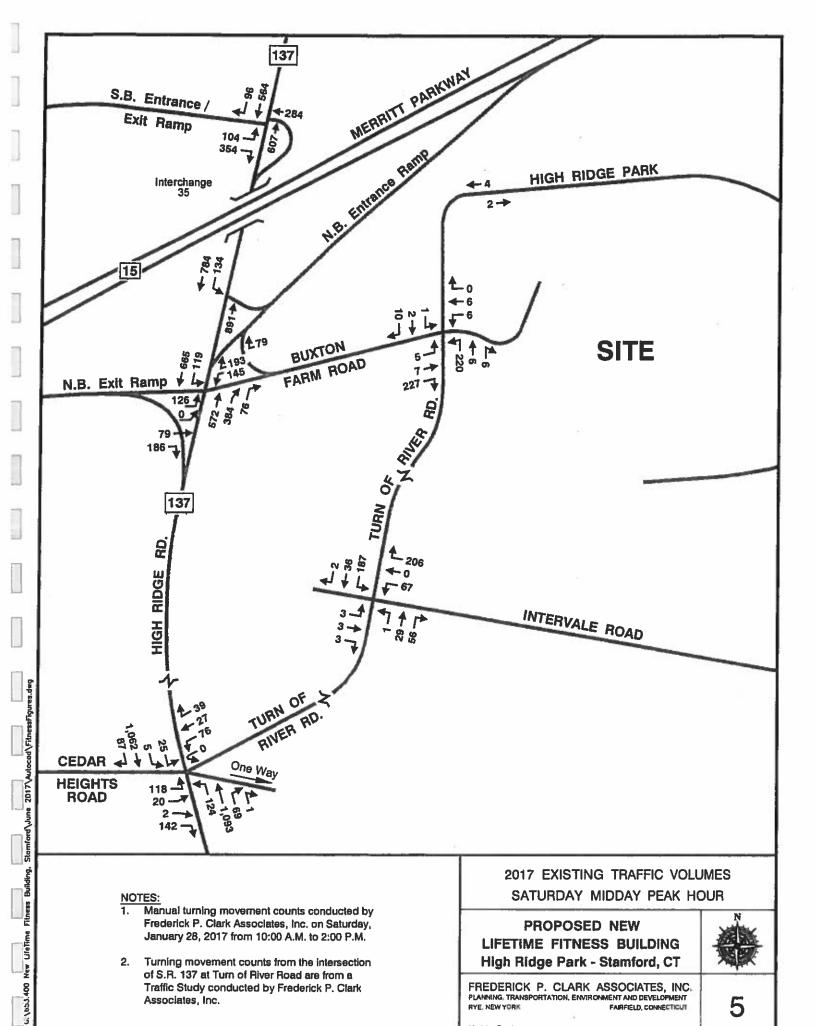
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S.R. 137 at Turn of River Road are from a Traffic Study

conducted by Frederick P. Clark Associates, Inc.





Not to Scale

Table 1 2017 TRAFFIC VOLUMES – PEAK HOURS Proposed New LifeTime Fitness Building High Ridge Park Stamford, Connecticut

	VELUCIES					
		VEHICLES				
LOCATION	Weekday	Weekday	Saturday			
LOCATION	Morning	Afternoon	Midday			
State Route 15 (Merritt Parkway) Southbound Ramps, West of	1,840	1,217	838			
State Route 137 (High Ridge Road)	1,040	1,217	000			
State Route 15 (Merritt Parkway) Southbound On-Ramp, East	553	463	284			
of State Route 137 (High Ridge Road)		403	204			
State Route 137 (High Ridge Road), North of State Route 15	2,364	2.042	4 274			
(Merritt Parkway) Southbound Ramps	2,304	2,042	1,371			
State Route 137 (High Ridge Road), South of State Route 15	3,118	0 777	4.000			
(Merritt Parkway) Southbound Ramps	٥,110 	2,777	1,809			
State Route 15 (Merritt Parkway) Northbound Off-Ramp, West	604	004	004			
of State Route 137 (High Ridge Road)	684	694	391			
State Route 15 (Merritt Parkway) Northbound On-Ramp, East	705	000	507			
of State Route 137 (High Ridge Road)	765	986	597			
Buxton Farm Road, East of State Route 137 (High Ridge	4 220	4.000	204			
Road)	1,339	1,328	691			
State Route 137 (High Ridge Road), North of State Route 15	0.440	0.777	4.000			
(Merritt Parkway) Northbound Ramps/Buxton Farm Road	3,118	2,777	1,809			
State Route 137 (High Ridge Road), South of State Route 15	0.000	2.22				
(Merritt Parkway) Northbound Ramps/Buxton Farm Road	2,866	2,887	2,028			
Assisted Living Access Drive, East of Turn of River Road	15	6	26			
Buxton Farm Road, West of Turn of River Road	1,167	1,101	475			
Turn of River Road, North of Buxton Farm Road/Assisted						
Living Access Drive	482	536	24			
Turn of River Road, South of Buxton Farm Road/Assisted						
Living Access Drive	986	895	467			
Turn of River Road, North of Intervale Road/Access Drive	992	877	463			
Turn of River Road, South of Intervale Road/Access Drive	383	380	192			
Intervale Road, East of Turn of River Road	1,044	848	519			
Access Drive, West of Turn of River Road	10	9	12			
State Route 137 (High Ridge Road), North of Cedar Heights			12			
Road/Turn of River Road	2,160	2,438	2,429			
State Route 137 (High Ridge Road), South of Cedar Heights						
Road/Turn of River Road	2,515	2,756	2,567			
Turn of River Road, East of State Route 137 (High Ridge						
Road)	373	375	256			

Cont'd Table 1

		VEHICLES	
LOCATION	Weekday Morning	Weekday Afternoon	Saturday Midday
Cedar Heights Road, West of State Route 137 (High Ridge Road)	461	706	520
Turn of River Road at High Ridge Park	481	524	6

Source:

- 1) Manual turning movement counts were conducted by Frederick P. Clark Associates, Inc. on Thursday, January 26, 2017, Thursday, March 9, 2017 and Saturday, January 28, 2017 for all intersections except High Ridge Road (State Route 137) at Turn of River Road and Cedar Heights Road.
- 2) High Ridge Road (State Route 137) at Turn of River Road and Cedar Heights Road volumes were from a Traffic Study prepared by this office.

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three-year period. Data indicates that each of the accidents were limited to property damage. The collision types involved a rear-end and an angle collision. The contributing factors were following too closely and failure to grant right-of-way. It was found that both of the accidents occurred during dark-lit conditions and on dry road conditions.

For the section of High Ridge Road between the Merritt Parkway Northbound Off-Ramp Right Turn and Merritt Parkway Northbound Ramps/Buxton Farm Road, there were a total of 4 accidents recorded during this three-year period. Data indicates that 75 percent of the accidents were limited to property damage and 25 percent included injuries. The collision types were 75 percent involving a rear-end collision and 25 percent involving turning intersecting path. The contributing factors were 75 percent for following too closely and 25 percent for failure to grant right-of-way. It was found that 75 percent of the accidents occurred during daylight hours and all of the accidents occurred on dry road conditions.

At the intersection of High Ridge Road at Merritt Parkway Northbound Ramps/Buxton Farm Road, there were a total of 33 accidents recorded during this three-year period. Data indicates that 79 percent of the accidents were limited to property damage and 21 percent included injuries. The collision types were 43 percent involving a rear-end collision, 27 percent involving an angle collision, 21 percent for sideswipe in the same direction, 6 percent involving a collision with a fixed object and 3 percent for turning in the opposite direction. The contributing factors were 43 percent for following too closely, 27 percent for failure to grant right-of-way, 9 percent involved an improper lane change, 6 percent for failure to stay in lane and 3 percent for no contributing action, other contributing action, an improper passing maneuver, an improper turning maneuver and the driver lost control. It was found that 70 percent of the accidents occurred during daylight hours and 79 percent occurred on dry road conditions.

High Ridge Road, between the Merritt Parkway Northbound Ramps/Buxton Farm Road and Merritt Parkway Northbound On-Ramp from Southbound Left Turn, had a total of 5 accidents recorded during this three-year period. Data indicates that all of the accidents were limited to only property

damage. The collision types were 80 percent involving a rear-end collision and 20 percent involved a sideswipe in the same direction. The contributing factors were 80 percent for following too closely and 20 percent were unknown. It was found that 60 percent of the accidents occurred during daylight hours and all of the accidents occurred on dry road conditions.

Data indicates that at the intersection of High Ridge Road at the Merritt Parkway Northbound On-Ramp from Southbound Left Turn, there was a total of one accident recorded during this three-year period. It indicates that the accident involved injuries, with the collision type being other. The contributing factor was an improper turning maneuver. It was found that the accident occurred during daylight hours and on wet road conditions.

In the section of High Ridge Road between the Merritt Parkway Northbound On-Ramp from Southbound Left Turn and Merritt Parkway Southbound On-Ramp Jug Handle, there were a total of 10 accidents recorded during this three-year period. Data indicates that all of the accidents were limited to property damage. The collision types were 30 percent involving rear-end and angle collisions, 20 percent involving sideswipe in the same direction and 10 percent for turning in the opposite direction and a collision with a fixed object. The contributing factors were 30 percent for failure to grant right-of-way, 20 percent for following too closely and 10 percent for failure to stay in lane, no contributing action, an improper passing maneuver, an improper lane change and ran off roadway. It was found that 80 percent of the accidents occurred during daylight hours and 90 percent of the accidents occurred on dry road conditions.

For the intersection of High Ridge Road at the Merritt Parkway Southbound On-Ramp Jug Handle, there was a total of one accident recorded during this three-year period. Data indicates that the accident involved injuries. The collision type was a rear-end collision, with a contributing factor of following too closely. It was found that the accident occurred during daylight hours and on wet road conditions. For the section of High Ridge Road, between the Merritt Parkway Southbound On-Ramp Jug Handle and Merritt Parkway Southbound On-Ramp, there were no recorded accidents.

At the intersection of High Ridge Road at the Merritt Parkway Southbound On-Ramp, there were a total of 12 accidents recorded during this three-year period. Data indicates that 83 percent of the accidents were limited to property damage and 17 percent included injuries. The collision types were 42 percent involving a rear-end collision, 17 percent involving an angle collision and collision with a moving object and 8 percent for turning intersecting path, sideswipe in the same direction and other collision. The contributing factors were 34 percent for following too closely, 26 percent for no contributing action and 8 percent for failure to grant right-of-way, other contributing action, an improper turning maneuver, violating a traffic control and improper backing. It was found that 50 percent of the accidents occurred during daylight hours and 83 percent occurred on dry road conditions. Table 2 provides a more detailed summary of the accident data.

For the section of Buxton Farm Road, between High Ridge Road and Turn of River Road, there were a total of three accidents recorded during this three-year period. Data indicates that all accidents were limited to property damage. The collision types were 34 percent involving a rear-end collision and 33 percent involved turning intersecting path and an angle collision. Contributing factors were 67 percent involving failure to grant right-of-way and 33 percent for following too closely. It was found that all of the accidents occurred during daylight hours, with 67 percent occurred on dry road conditions.

At the intersection of Buxton Farm Road at Turn of River Road, there was a total of one accident recorded during this three-year period. Data indicates that the accident involved injuries. The collision type was an angle collision. The contributing factor was failure to grant right-of-way. It was found that the accident occurred during daylight hours and on wet road conditions.

On the section of Turn of River Road, between Buxton Farm Road and Intervale Road, there were a total of two accidents recorded during this three-year period. Data indicates that the accidents involved both property damage and injuries. The collision types were with a fixed object. The contributing factors were driver lost control and ran off roadway. It was found that all of the accidents occurred during daylight hours and on dry road conditions.

Table 2
ACCIDENT EXPERIENCE SUMMARY – STATE ROUTE 137
Proposed New LifeTime Fitness Building
High Ridge Park
Stamford, Connecticut

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			rear - 2044		• 2016 Total	Accident	PropeInjury	Collision		- Tumin		- Angle		• Movin	Contribut	Follow	Failur	• No Co	Other	Monday .		Driver	- Improp	 Violate 	- Ran O	Impro	

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Source: 1) 2)

Connecticut Department of Transportation from January 1, 2014 to December 31, 2014. Connecticut Crash Data Repository from January 1, 2015 to December 31, 2016.

-2

Notes:

January 1, 2014 to December 31, 2016 is the latest three full years of accident data available.

The 2015 and 2016 accident data follows the new Connecticut Uniform Police Report. The collision type was determined using the manner of crash/collision impact and the accident diagram. For weather conditions, no adverse was

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At the intersection of Turn of River Road at Intervale Road, there were a total of two accidents recorded during this three-year period. Data indicates that the accidents involved both property damage and injuries. The collision types involved a rear-end and an angle collision. The contributing factors were no contributing action and ran STOP sign. It was found that 50 percent of the accidents occurred during daylight hours and all of the accidents occurred on wet road conditions. Table 3 provides a more detailed summary of the accident data. The accident data obtained from both ConnDOT and the Connecticut Crash Data Repository is included in the Appendix of this report.

Table 3 ACCIDENT EXPERIENCE SUMMARY -- BUXTON FARM ROAD/TURN OF RIVER ROAD Proposed New LifeTime Fitness Building High Ridge Park Stamford, Connecticut

	B	UXTON FAF	RM ROAD		TUE	N OF RI	VER ROA	D
	Between S		1111110710		Between			
		and Turn of River		of River	Farm Ro		At Inte	ervale
	Ro		Ro		Intervale		Ro	
ACCIDENT CHARACTERISTICS	Total	%	Total	%	Total	%	Total	%
Year								
2014	2	67	1	100	1	50	0	0
= 2015	1	33	0	0	0	0	2	100
= 2016	1 0	0	0	0	1	50	0	0
■ Total	3	100	1	100	2	100	2	100
Accident Severity								
Property Damage	3	100	0	0	1	50	1	50
= Injury	0	0	1	100	1	50	1	50
Collision Type								27.1
Rear-end	1	34	0	0	0	0	1	50
 Turning-Intersect Path 	1	33	0	0	0	0	0	0
Angle	1	33	1	100	0	0	1	50
Fixed Object	0	0	0	0	2	100	0	0
Contributing Factor					<u> </u>			
Following Too Closely	1 .	33	0	0	0	0	0	0
 Failure to Grant ROW 	2	67	1	100	0	0	0	0
 No Contributing Action 	0	0	0	0	0	0	1	50
Ran STOP Sign	0	0	0	0	0	0	1	50
 Driver Lost Control 	0	0	0	0	1	50	0	0
Ran Off Roadway	0	0	0	0	1	50	0	0
Light Condition								
Daylight	3	100	1	100	2	100	1	50
Dusk	0	0	0	0	0	0	1	50
Surface Condition								
■ Dry	2	67	0	0	2	100	0	0
■ Wet	1	33	1 .	100	0	0	2	100
Weather Conditions								
No Adverse	2	67	0	0	1	50	1	50
Clear	0	0	0	0	1	50	0	0
Rain Rain	1	33	1	100	0	0	1	50

Source:

- 1) Connecticut Department of Transportation from January 1, 2014 to December 31, 2014.
- 2) Connecticut Crash Data Repository from January 1, 2015 to December 31, 2016.

Notes:

- 1) January 1, 2014 to December 31, 2016 is the latest three full years of accident data available.
- 2) The 2015 and 2016 accident data follows the new Connecticut Uniform Police Report. The collision type was determined using the manner of crash/collision impact and the accident diagram. For weather conditions, no adverse was replaced with clear.

FUTURE TRAFFIC IMPACTS

This section of the report provides a description of the future no-build traffic volumes for a 2019 condition, site-traffic generation, site traffic assignment and future build traffic volumes.

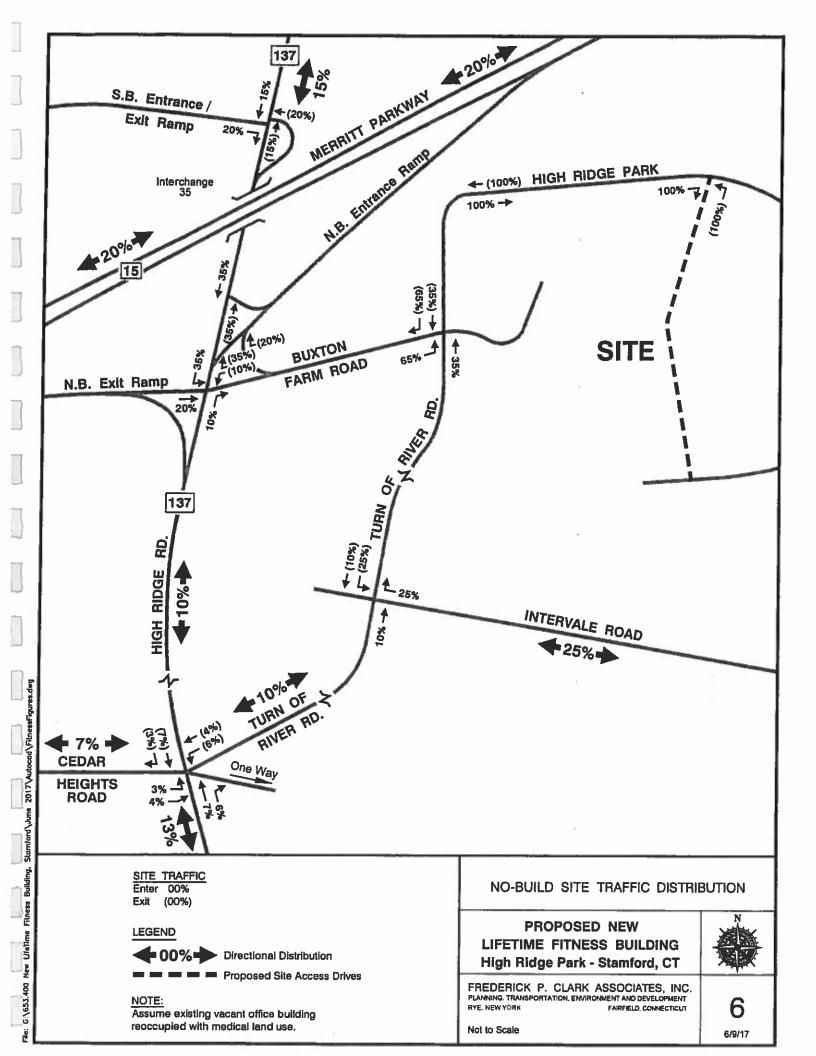
No-Build Traffic Volumes

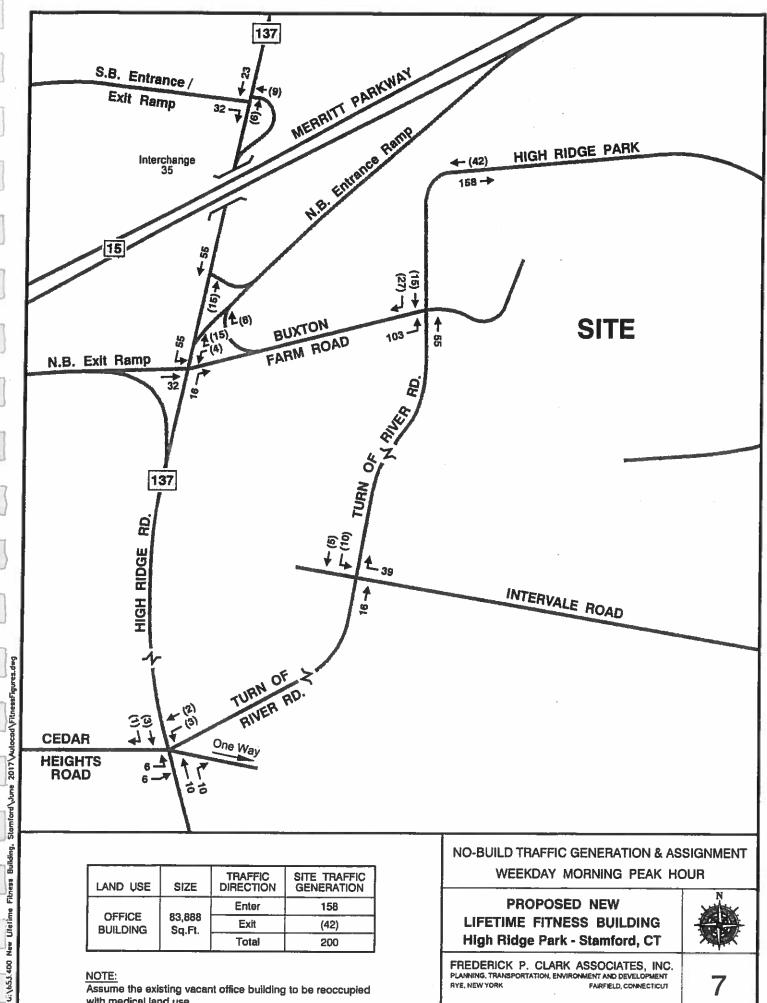
Future no-build traffic volumes, without the proposed development, assume that the currently vacant Building 3 will be reoccupied with medical uses (because current market trends indicate that this would be the most likely use group). This traffic generation is based on trip generation rates provided by ITE. It is estimated that a total of 200, 299 and 305 vehicle trip ends will be added to the study area intersections during the weekday morning, weekday afternoon and Saturday midday peak hours, respectively.

Based on a review of current traffic patterns at Study Area intersections and guidance from ConnDOT, a distribution plan was developed for the re-occupancy of Building 3 with medical uses and the removal of office space. It was determined that 20 percent of the site traffic will arrive from and depart to the east and west (each way), respectively, on State Route 15, 25 percent arrive from and depart to the east on Intervale Road, 15 percent will arrive from and depart to the north on High Ridge Road, 13 percent will arrive from and depart to the south on High Ridge Road and 7 percent will arrive from and depart to the west on Cedar Heights Road.

Figure 6 graphically illustrates the patterns assumed and used for the reoccupancy of Building 3 with medical uses. Figures 7 through 9 illustrate the traffic generation and assignment for the reoccupancy of Building 3 with medical uses during the weekday morning, weekday afternoon and Saturday midday peak hours, respectively.

The 2017 traffic volumes were expanded to reflect a 2019 traffic condition by applying a 0.6 percent per year growth rate, as per discussions with the ConnDOT Planning Division. Based on discussions with both the City of Stamford Planning Department and the ConnDOT Planning





LAND USE	SIZE	TRAFFIC DIRECTION	SITE TRAFFIC GENERATION
955195		Enter	158
OFFICE BUILDING	83,888 Sq.Ft.	Exit	(42)
		Total	200

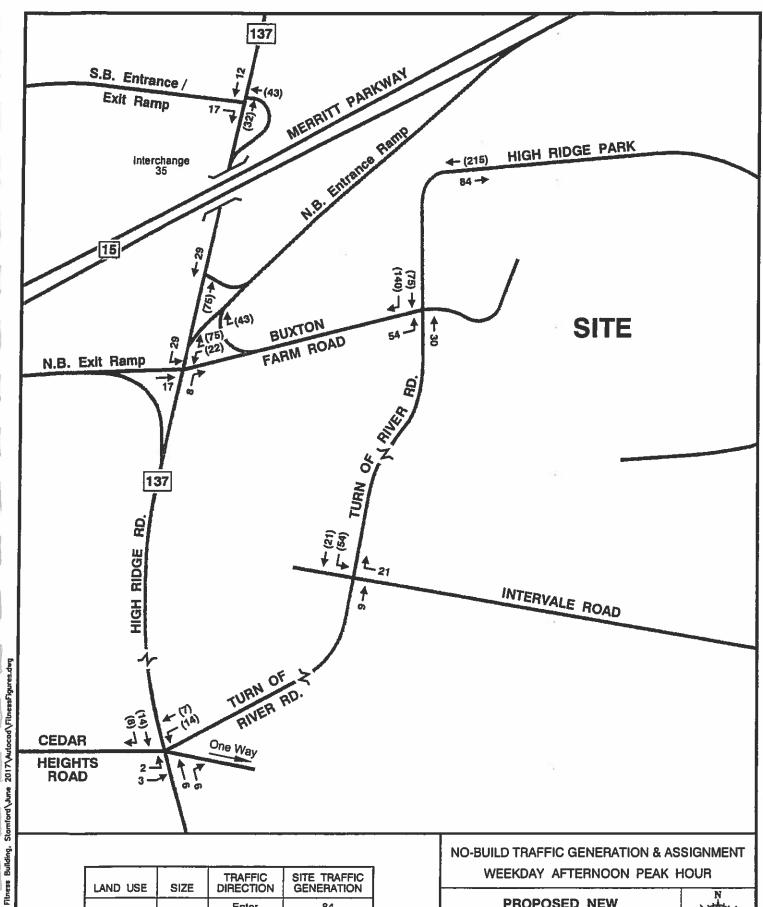
Assume the existing vacant office building to be reoccupied with medical land use.

PROPOSED NEW LIFETIME FITNESS BUILDING High Ridge Park - Stamford, CT



FREDERICK P. CLARK ASSOCIATES, INC. PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT RYE, NEW YORK FARFIELD, CONNECTICUT FAIRFIELD, CONNECTICUT

Not to Scale



LAND USE	SIZE	TRAFFIC DIRECTION	SITE TRAFFIC GENERATION		
		Enter	84		
OFFICE BUILDING	83,888 Sq.Ft.	Exit	(215)		
55.25.110	- waq., t.	Total	299		

New LifeTime

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Assume the existing vacant office building to be reoccupied with medical land use.

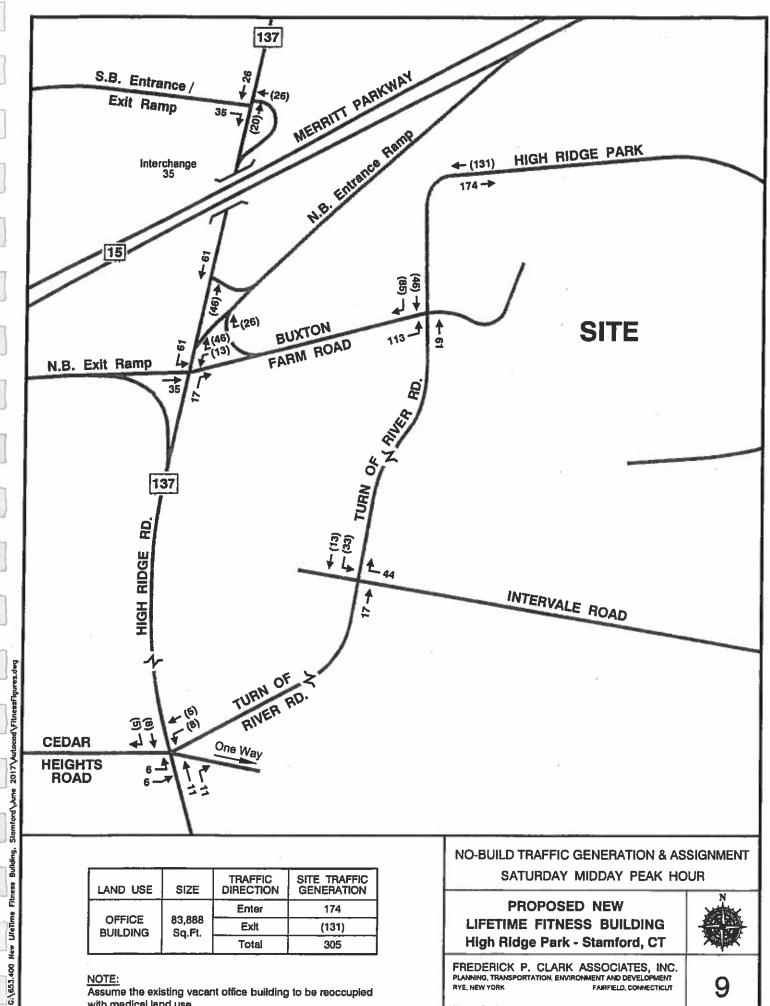
PROPOSED NEW
LIFETIME FITNESS BUILDING
High Ridge Park - Stamford, CT



FREDERICK P. CLARK ASSOCIATES, INC. PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT RYE. NEW YORK FAIRFIELD, CONNECTICUT

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LAND USE	SIZE	TRAFFIC DIRECTION	SITE TRAFFIC GENERATION
075105		Enter	174
OFFICE BUILDING	83,888 Sq.Ft.	Exit	(131)
		Total	305

NOTE:

Assume the existing vacant office building to be reoccupied with medical land use.

LIFETIME FITNESS BUILDING High Ridge Park - Stamford, CT



FREDERICK P. CLARK ASSOCIATES, INC. PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT RYE, NEW YORK FAIRFIELD, CONNECTICUT FAIRFIELD, CONNECTICUT

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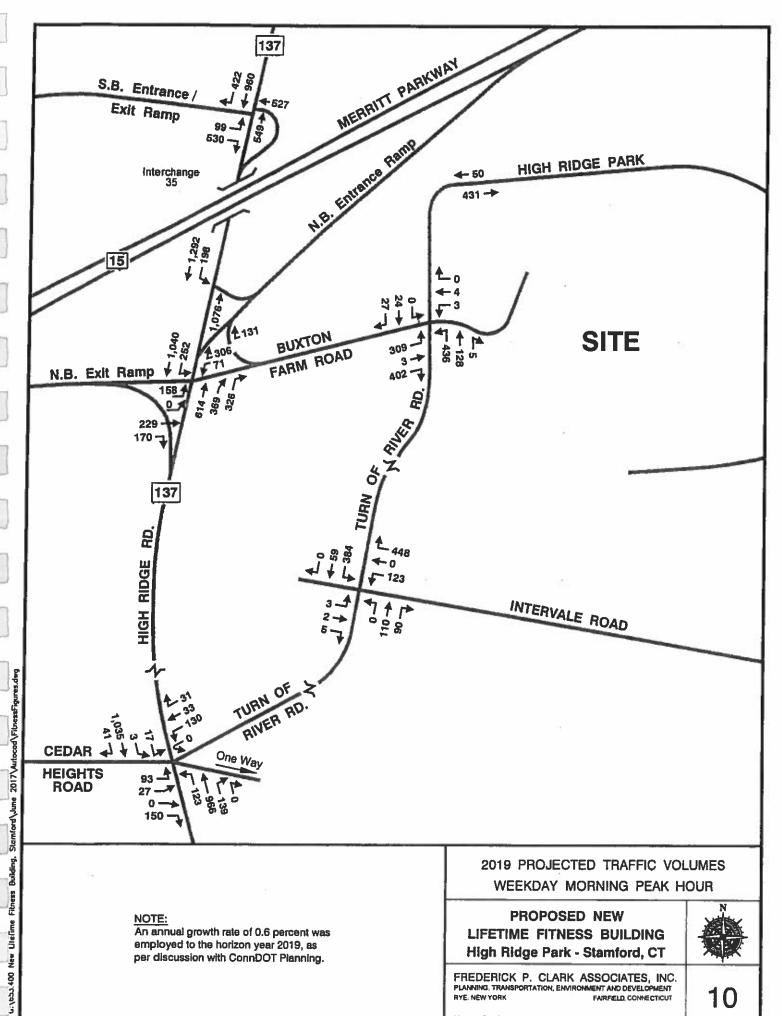
Department, no other developments were identified for inclusion in this analysis. Figures 10 through 12 graphically illustrate the 2019 projected traffic volumes for the weekday morning, weekday afternoon and Saturday midday peak hours, respectively.

The 2019 no-build traffic volumes were then determined by adding the traffic for the reoccupancy of Building 3 with medical uses to the 2019 projected traffic volumes. Figures 13 through 15 graphically illustrate the 2019 no-build traffic volumes for the weekday morning, weekday afternoon and Saturday midday peak hours, respectively.

Site Traffic Generation

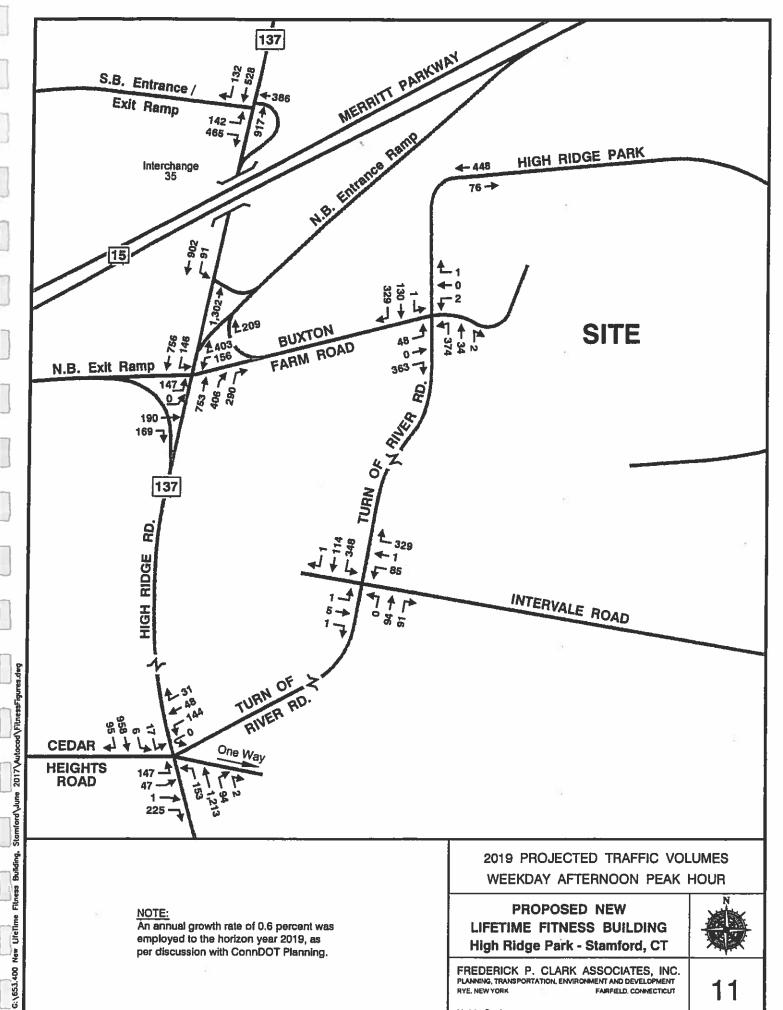
This analysis assumes that the existing office Building 3 is re-occupied with a medical land use and then replaced with an 114,000 square-foot LifeTime Fitness Building and that approximately 6,128 square-feet of office space from the existing office park facility is converted to storage. It is anticipated that the proposed Fitness Center will generate a total of 161, 402 and 317 total trip ends of which 56, 141 and 0 are internal capture trips (as approved by ConnDOT) and a total of 105, 261 and 317 external vehicle trips during the weekday morning, weekday afternoon and Saturday midday peak hours, respectively. The office space to be removed is anticipated to generate a total of 10, 10 and 3 vehicle trip ends during the weekday morning, weekday afternoon and Saturday midday peak hours, respectively. The internal trips are between the proposed LifeTime Fitness Building and the existing office park during the weekday morning and weekday afternoon peak hours of the adjacent street system.

The net change between Building 3, fully occupied with medical uses coupled with the removal of 6,128 square-foot office space from the existing office park, and the proposed 114,000 square-foot LifeTime Fitness Building is a reduction of 105 and 48 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. During the Saturday midday peak hour there would be a very modest increase of 9 vehicle trip ends. Table 4 provides a more detailed summary for the re-occupancy traffic, proposed site traffic generation and net difference during the peak hours.

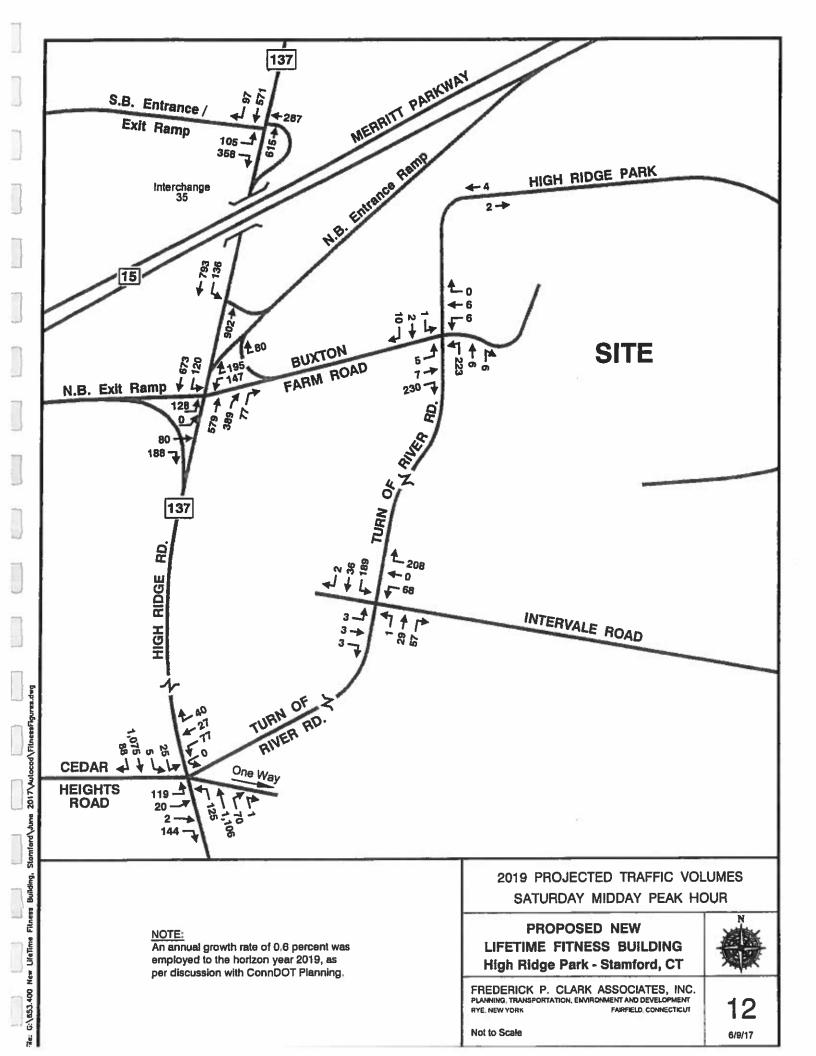


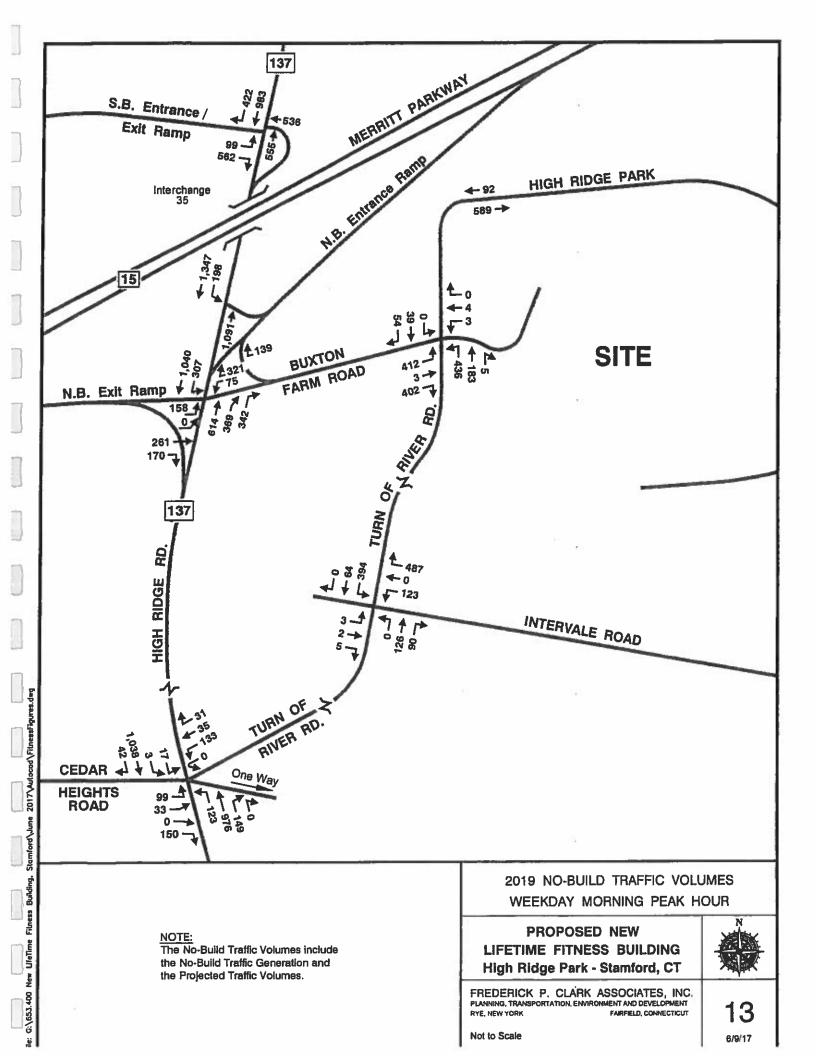
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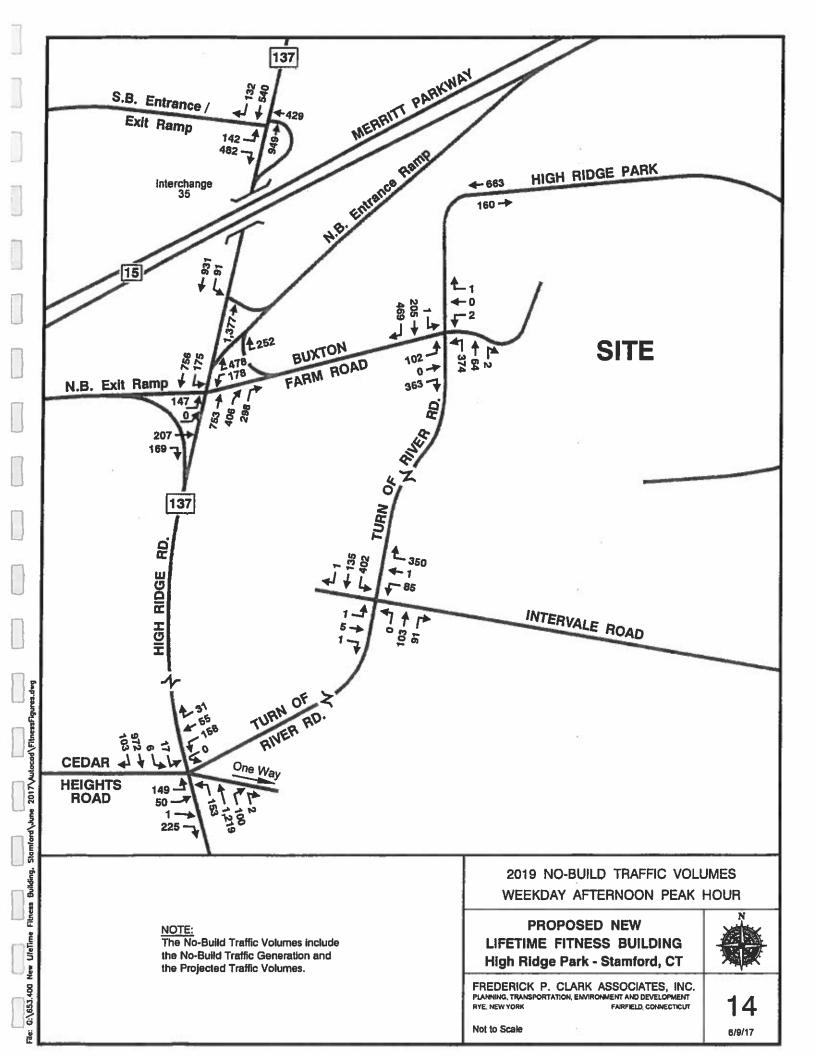
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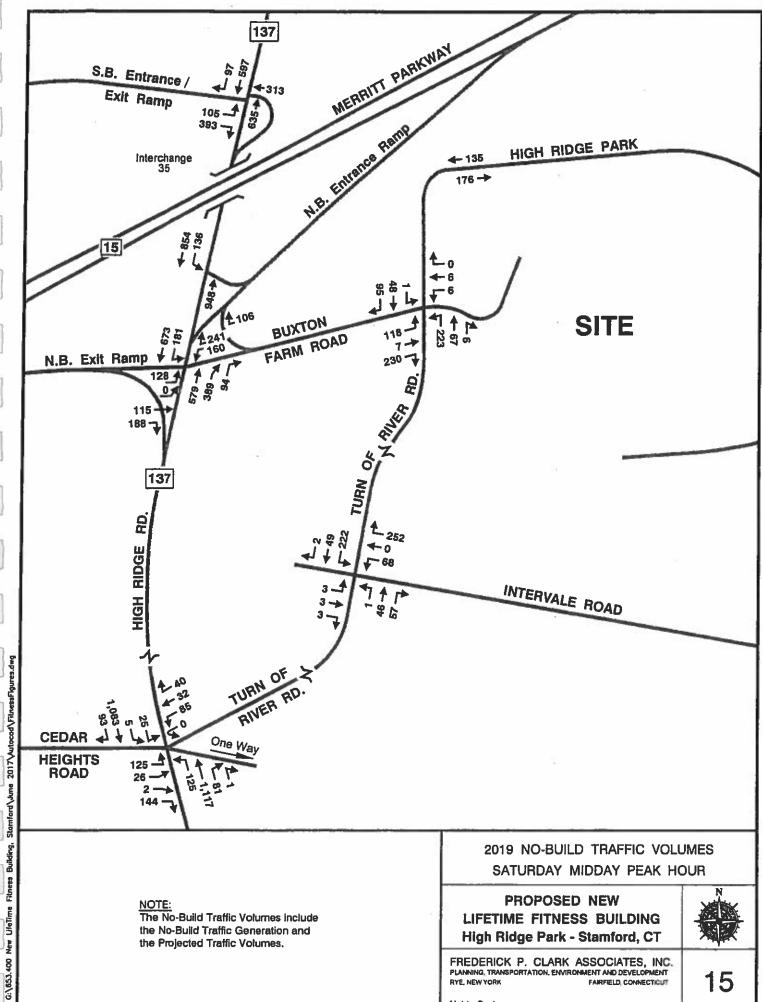


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Table 4
SITE TRAFFIC GENERATION FORECAST – PEAK HOURS
Proposed New LifeTime Fitness Building
High Ridge Park
Stamford, Connecticut

					SITI	TRAFFIC G	SITE TRAFFIC GENERATION AND ADJUSTMENT	AND ADJUS	TMENT		
			E	Total Trin Fnde		Inter	Internal Canture (35%)	(%)	7	External Vehicle Trin Ends	13 13 14
		לוביד 4 חיד		1 - 1		111100	210000000000000000000000000000000000000		LAKEIII	מו אכוווכוכ זווו	CINC
		IKAFFIC	Weekday	Weekday	Saturday	Weekday	Weekday	Saturday	Weekday	Weekday	Saturday
LAND USE	SIZE	DIRECTION	Morning	Afternoon	Midday	Morning	Afternoon	Midday	Morning	Австооп	Midday
1 - Assume Existing Office	83,888 S.F.	Enter	-158	-84	-174	1	ı	1	-158	-84	-174
Building to be Occupied		Exit	42	-215	-131	1)	11	11	-42	-215	-131
with Medical Land Use		Total	-200	-299	-305	:	ı	ı	-200	-299	-305
(ITE Code #720)											
2- General Office Space to	6,128 S.F.	Enter	6-	-2	-2	1	:	:	6-	-2	-2
be Removed		Exit	7	ᅇ	7	ı	i	ı	-	00	7
(ITE Code #710)		Total	-10	-10	-3	-	:	1	01-	01-	ائ
3-Total vehicle trip ends to	90,016 S.F.	Enter	-167	98-	921-	1	•	,	-167	98-	-176
be Removed from Study		Exit	43	-223	-132	ı		:	43	-223	-132
Area (1+2)		Total	-210	-309	-308	ı		-	-210	-309	-308
4 - Proposed LifeTime	114,000 S.F.	Enter	81	229	143	28	80	0	53	149	143
Fitness Building		Exit	<u></u>	173	174	28	19	0	52	112	174
(ITE Code #492)		Total	161	402	317	56	141	0	105	261	317
Net Difference	23,984 S.F.	Enter	98-	143	-33	28	80	0	-114	63	-33
		Exit	37	- 20 21	42	28	<u>61</u>	0	6	-111	42
		Total	-49	93	6	56	141	0	-105	-48	6

Source:

1) The Institute of Transportation Engineers (ITE), Trip Generation Manual 9th Edition, 2012 using Medical-Dental Office Building, Code #720 Average Rates, General Office, Code #710 and Health/Fitness Club, Code #492 Average Rates. Note: Internal Capture: Based on a discussion with Connecticut Department of Transportation, Bureau of Policy and Planning, a 35 percent credit was employed to the total trip ends to account for members using the facility that work in the Office Park. No internal capture is taken for the Saturday midday peak hour.

Frederick P. Clark Associates, Inc. G. 1833 400 New LifeTime Fitness Building, StanfordNune 2017\word\till 17.004.mex.doc 6/12/17

Distribution and Assignment of Site-Generated Traffic

Based on a review of current traffic patterns at the Study Area intersections, anticipated travel routes to the site and guidance from ConnDOT, a distribution plan was developed. As described above, the site access drives will be to the internal High Ridge Park driveway. It was determined that 20 percent of the site traffic will arrive from and depart to the east and west (each way), respectively, on State Route 15, 25 percent arrive from and depart to the east on Intervale Road, 15 percent will arrive from and depart to the north on High Ridge Road, 13 percent will arrive from and depart to the south on High Ridge Road and 7 percent will arrive from and depart to the west on Cedar Heights Road.

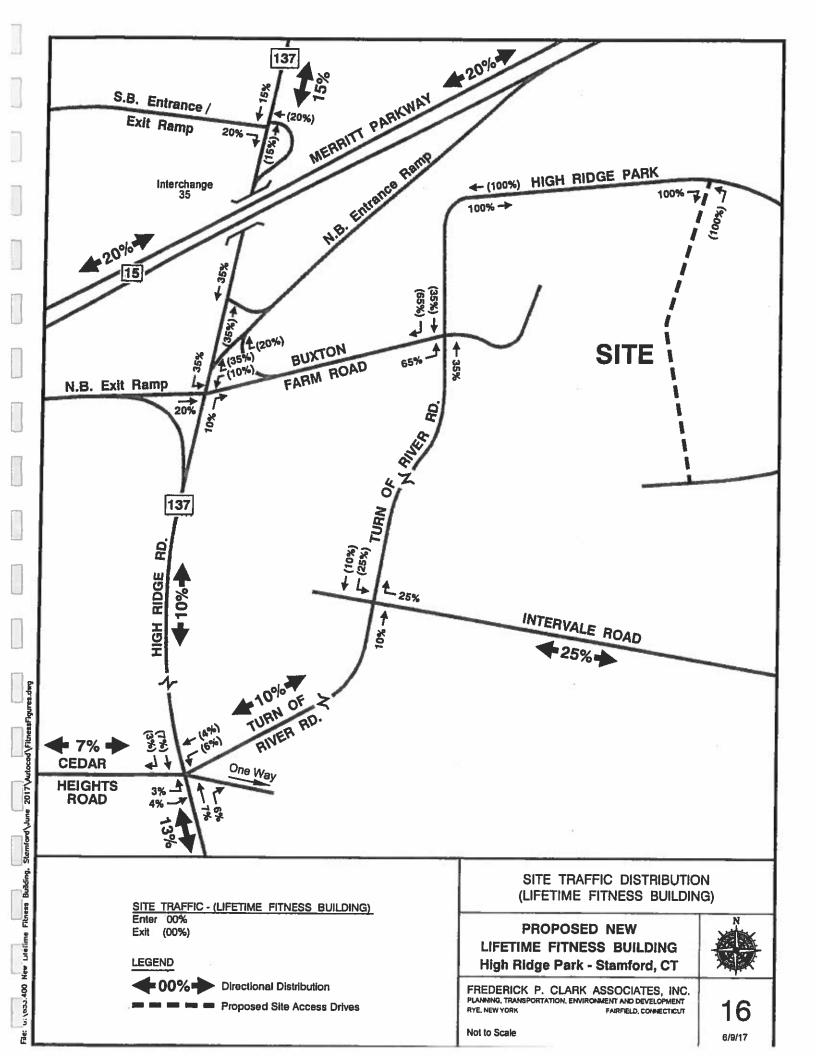
Figure 16 graphically illustrates the patterns assumed and used for the LifeTime Fitness Building traffic. Figures 17 through 19 illustrate the net difference in traffic generation and assignment between the reoccupied Building 3 with medical uses and LifeTime Fitness Building during the weekday morning, weekday afternoon and Saturday midday peak hours, respectively.

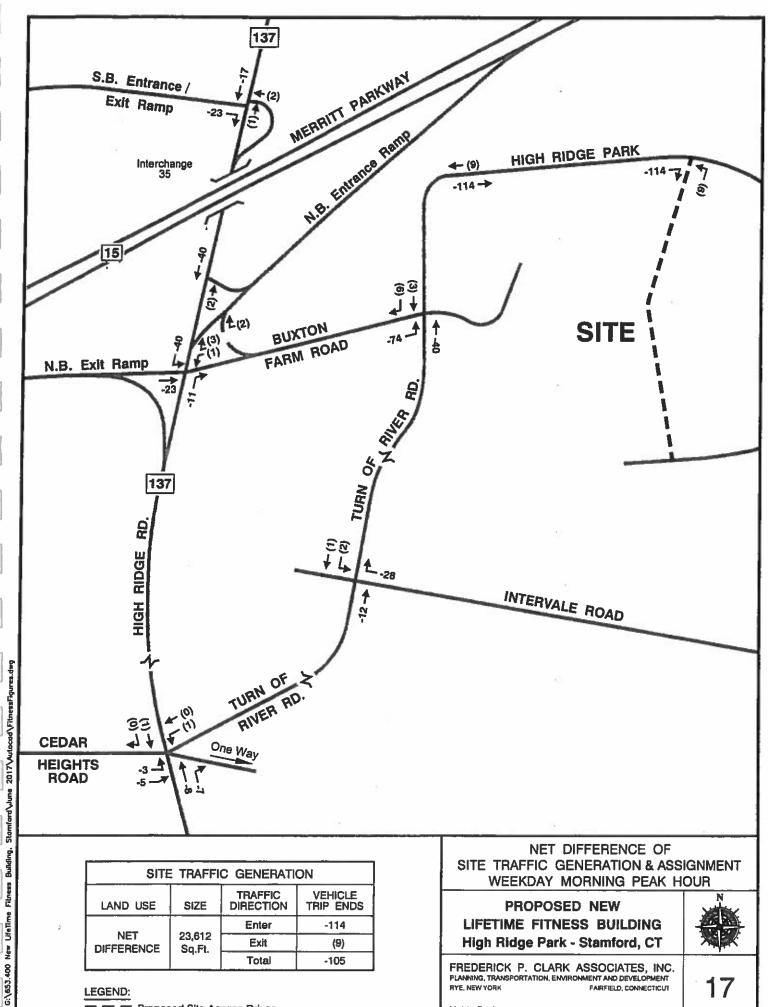
Build Traffic Volumes

Build traffic volumes were developed based on adding the net difference in traffic generation and assignment between the reoccupied Building 3 with medical uses and LifeTime Fitness Building to the no-build traffic volumes described above for a 2019 design year. Figures 20 through 22 graphically illustrate the build traffic volumes for the weekday morning, weekday afternoon and Saturday midday peak hours, respectively.

Capacity Analysis Procedures

Capacity analysis procedures are provided in the Appendix of this report. The analyses follow a SYNCHRO computer model and information provided by the Transportation Research Board (TRB) and the Highway Capacity Manual (HCM) published in 2010.



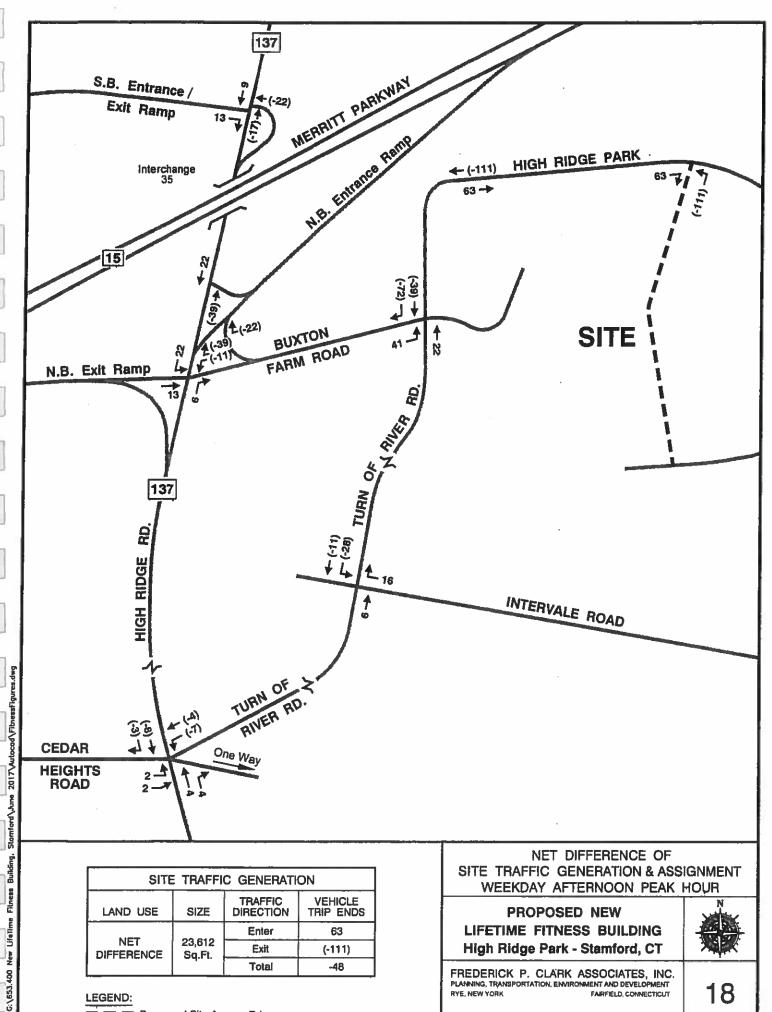


LEGEND:

Proposed Site Access Drives

PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT RYE. NEW YORK FAIRFIELD, CONNECTICUT

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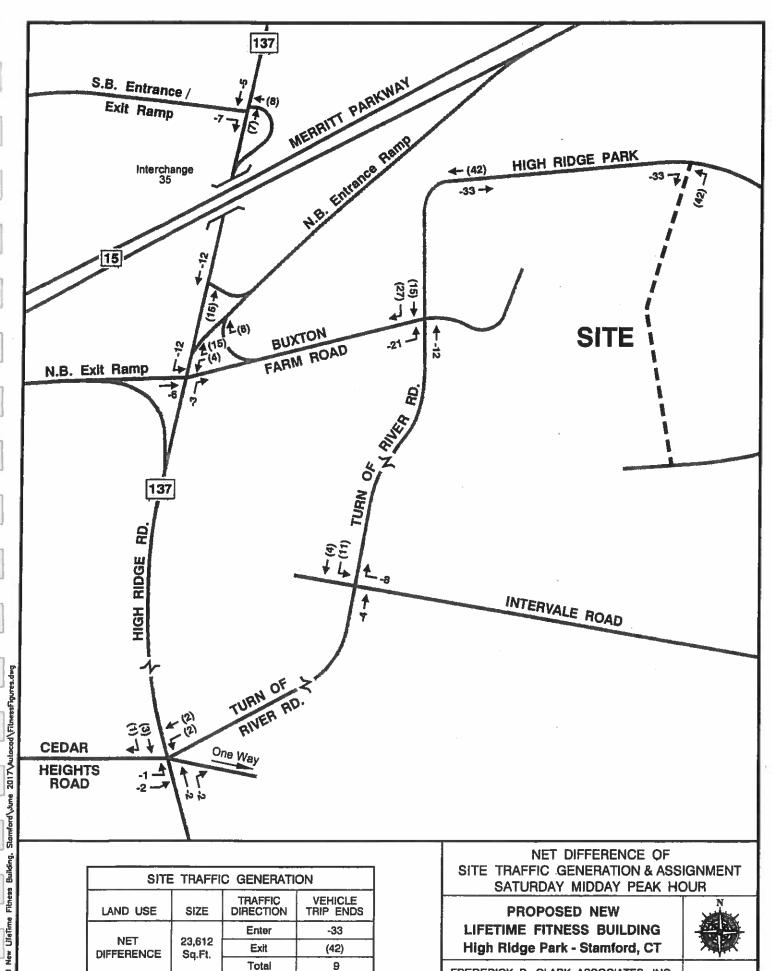
LEGEND:

Proposed Site Access Drives

RYE. NEW YORK FAIRFIELD, CONNECTICUT

18

Not to Scale



EGEND:

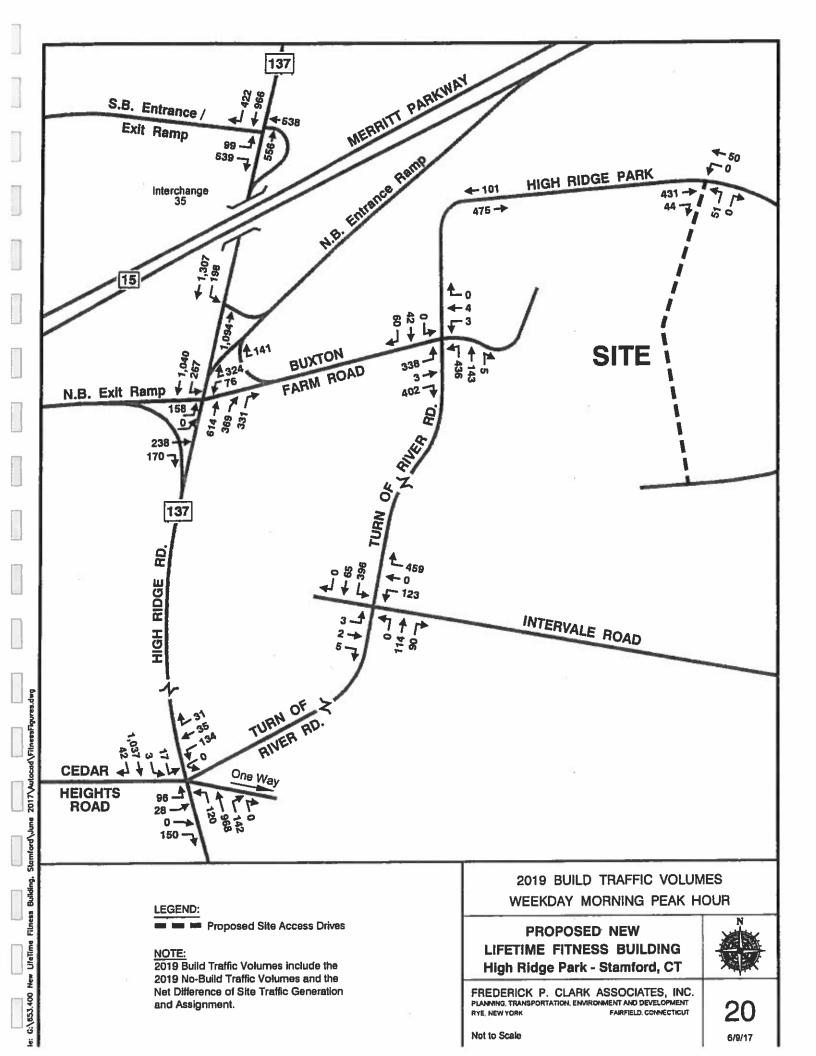
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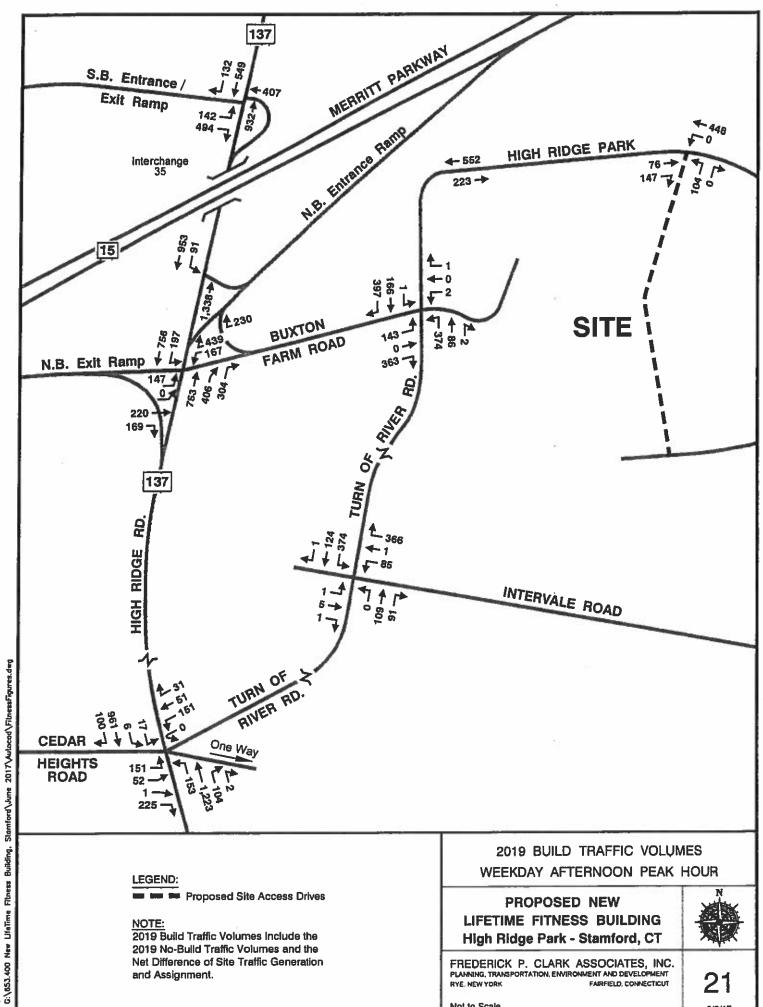
Proposed Site Access Drives

FREDERICK P. CLARK ASSOCIATES, INC. PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT RYE. NEW YORK FAIRFIELD, CONNECTICUT

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Not to Scale

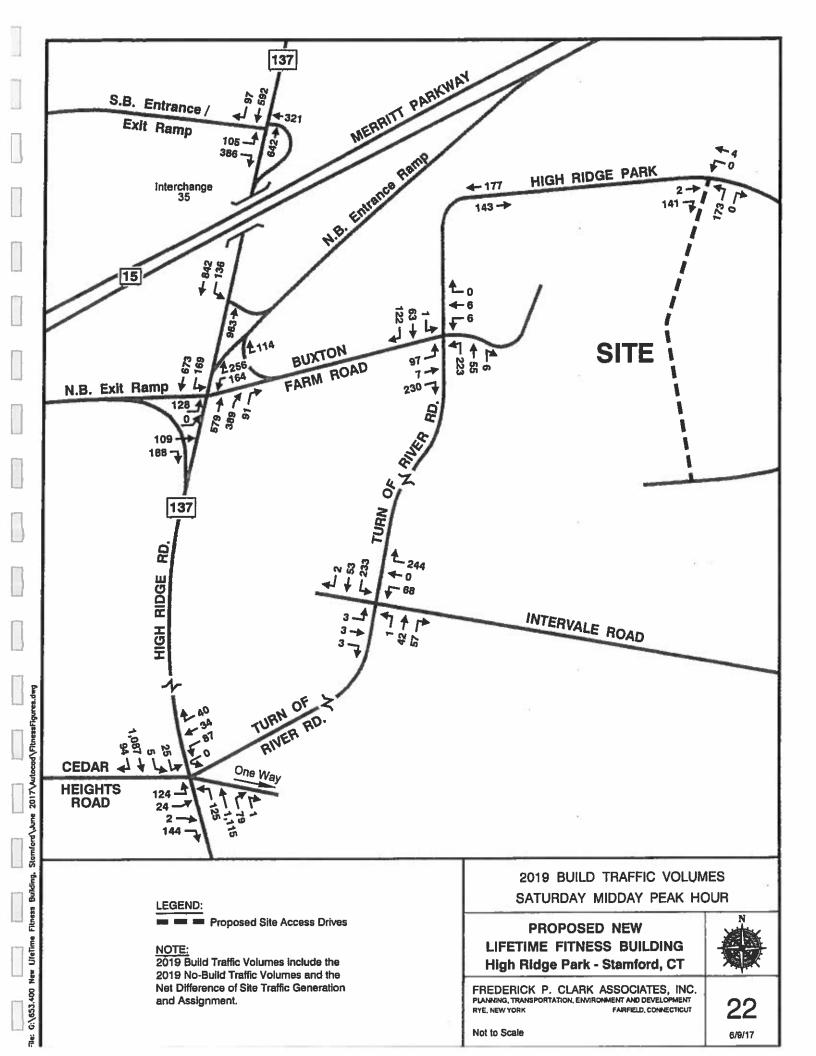




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Capacity Analysis Results – Existing, No-Build and Build Conditions

To determine the potential impact of area roadways, capacity analyses were completed for Study Area intersections, as well as the site access drive. The comparison between a no-build and build condition provides the potential impact from the proposed development. As previously described, the proposed development will result in a net decrease in traffic impact when compared to no build conditions. Results of the capacity analysis and the storage/queue analysis indicate that the Study Area intersections will continue to operate with oversaturated conditions during the Study Area peak hours. However, this condition is not exacerbated by the proposed development. In fact, in some instances, conditions are improved, and the construction of the proposed development will result in a net decrease in traffic impact during peak hours when compared to the permitted office use on the Campus

Tables 5 and 6 provide a more detailed summary of the results of the analyses, as described above. These tables provide Level of Service, average vehicle delay and volume to capacity ratio for each lane group, approach, movement, lane and intersection overall during each of the peak hours for an existing, no-build and builds condition. It also provides a comparison between the no-build and build conditions, which identifies the potential impact (if any). Tables 7 and 8 provide a more detailed summary of the results of the Storage/Queue analyses for each lane group, movement and lane during each of the peak hours for an existing, no-build and builds condition. The capacity worksheets are included in the Appendix of this report.

Findings

A Traffic Access and Impact Study was prepared to provide the City of Stamford, Connecticut Department of Transportation (ConnDOT) and the Office of the State Traffic Administration (OSTA) with a detailed analysis to determine potential traffic impacts from the proposed LifeTime Fitness Building. The site is located in the westerly side within the High Ridge Park. The proposed 114,000 square-foot Fitness Center will replace the existing 83,888 square-foot Building 3 and approximately 6,128 square-feet of office space from the office park facility, which will be converted to storage space.

Table 5
CAPACITY ANALYSIS RESULTS FOR SIGNALIZED INTERSECTIONS - MEASURE OF EFFECTIVENESS (MOE) AND IMPACT ASSESSMENT -- PEAK HOURS
Proposed New LifeTime Fitness Building
High Ridge Park
Slamford, Connecticut

_	1	-		- 1	_		_			_	_								_				_	_				_	_		_	_	_					
	Sahinday Midday	Pminn	Delay	Seconds	0.0	-O.3	-0.2	-0.5	-0.5	50.	0.1	0.1	6	6	0.4	-29	-20	70.7	0.5	0.0	0.3	-23	-23	0.2	0.2	-1.6	0.1	0.3	0.5	0.2	03	0.3	0.1	Ģ	0.0	0.7	0.1	000
OBUILD	Sahinda		Change	S I	2 :	Q :	운 :	S S	ટ્ટ	£	Ş	&	B-A	2	2	2	2	2	2	몬	ž	2	욷	욷	2	No	Se.	£	2	€ :	£	욷	ž	ક	ş	운	ટ	8 8
PROJECT SMPACTS (NO BUILD TO BUILD)	Affermon	Project	Delay	2 n	27.	2.5	9.5	1,0	0.	0.1	0.1	0.3	-0.3	0.0	-33.0	-57.6	47.6	-0.2	5.5	0.0	3.5	31.6	31.6	-0.4	-0.4	9.1	0.3	-0.4	-0.2	2.7	-0.2	4:	40.4	Ą	6	-0.2	-0.5	-24
IMPACTS (Weekday Aftermon		Change	3 9	2 .	9 : Y	운 :	£	욷	ş	ટ્ટ	2	운	£	2	2	ž	2	2	2	9₹	2	운	욷	<u>8</u>	용	2	2	운 :	2	2	£	2	운	£	욷	£	2 8
PROJECT	Momina	Project	Delay	16.6	0.0	9 0	0.6	20	တ်	21	21	<u></u>	-1:3	-3.6	0.0	-37.0	-30.1	0.7	23.7	0.0	16.7	6	£.	0.7	0.7	-10.0	-0.2	-0.2	-	5,5	0.0	0.0	Ó.	63	-0.2	0.2	-0.2	0 Q
	Weekday Momino		Change	3 5	2 5	₹ .	J .	윤 :	2	9-C	9-C	£	2	운	£	£	ટ	£	D-E	운	0-D	£	2	욷	S	٤	2	물 :	문 :	오 :	운 :	2	£	욷	C-B	A-B	몬	2 2
	ye ×		S Vic	250	5 2	25.0	1 6	7/1	1	0.40	1	0.44	1	1	0.46	0.85	1	0.46	0.46	0.07	1	1.40	1	0.64	1	ı	0.37	- F.	1 8	3 5	9	1	0.47	0.65	1	0.12	0.72	1 1
SNS	Saturday Midday		10S/	257	200		0.00	D/44.B	0/44.8	A7.5	N.5	B/15.6	A15.6	B/16.6	D/37.4	0/48.6	D/45.2	C722.2	C/223	A0.1	8/17.6	FI214.3	F/214.3	8/15.7	8/15.7	F/93.9	C/29.8	047.9	0.3	100	A4.8	6/18.1	B/13.6	B/18.9	By18.4	A9.1	22	C20.8
ONDITIC	lay		N.C.	880	0.46	?	1 8	1970	ı	0.57	ı	0.42	1	1	0.99	220	ı	0.36	0.80	0.16	1	223	1	0.75	,	1	0.40	 80 	1 8	0.30	0.24	1	0.51	0.72	1	0.11	69.0	1 1
2019 BUILD CONDITIONS	Weekday		LOSV	DV41 A	140 E	2000	500	200	C726.9	C24.5	C/24.5	BV18.3	BV18.3	C23.5	F/117.5	F/581.2	F/453.9	C/24.3	D/40.2	A0.2	C/26.0	F/580.9	F/580.9	C/20.3	C/20.3	F/299.4	C/32.2	200	E00.4	25.5	8 . G	C24.5	B/17.3	C24.2	C23.5	B/12.2	290	C224.5
201	day ng		S S	3 2	3 5	3	1 6	3	1 }	98.0	ı	0.92	1	1	0.61	1.43	1	0.53	0.92	0.10	1	1.28	1	0.83	ı	1	0.27	7) 2)	1 6	7 5	<u>0</u>	1	0.44	300	ı	8	0.73	l t
	Weekday		LOSV Delay	F/86.2	1000	Die in	0.15		1341.5	C/2/1.0	C221.D	0.35.6	D/35.6	D/37.3	D/529	F/242.2	F/189.3	D/45.7	E/69.5	A/0.1	248.1 1.9	F/140.6	F/140.6	C22.1	C7221	F/93.1	C225.4	7,947	CAUZ	9777	2.0	B/18:0	B/14.8	C203	8/19.8	B/10.1	C.24.9	C/24.7
	Midday		Ratio Sation	120	040	9	120	2.5	ı	D.40	1	0.44	ı	,	0.46	0.87	1	0.46	0.43	0.07	ı	1.40	ı	0.64	ı	1	0.37	20.0	1 8	200	0.0	ı i	0.46	0.65	ı i	0.12	0.72	1 1
2019 NO-BUILD CONDITIONS (BASE)	Saturday Midday		LOS/ Delay	B/12.7	P/10 7	7117	7.48.3	3.5	D/45.3	W/.b	A7.6	B/15.7	B/15.7	B/16.7	D/37.0	0/51.5	D/47.2	C/22.4	C/21.8	A0.1	BV17.4	F/216.6	F/216.6	B/15.5	B/15.5	1,05.5	25.23	0.45.0	0.000	7 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	A4.0	871/18	B/13.5	B/19.0	B/18.4	A90	22	C 28 8
SELECTION	<u> </u>		Saffic Saffic	8	0.45	5	200	70.0	ı	5	1	0.42	ŧ	١	10	234	ı	0.37	0.87	0.18	ı	2.16	ì	0.74	1	1	\$ 8 8 8	23	1 6	2 6	7	1	0.52	0.72	ı i	1.0	U.V	1 1
BUILD CO	Weekday		LOS.	D/43.4	A/9 6	סאמיט	250	0.00	200	C/24.4	C.24.4	B/18.5	B/18.6	C/23.5	F/150.5	F/638.8	F/501.5	C/24.5	D/46.7	A/0.2	C23.5	F/549.3	F/549.3	C220	C/28/1	1230.3	C31.9	9 2	0.00	5 0	0.77	(23)	B/17.7	C/24.3	67236	B/12.4	7.77	C/26.9
19 NO-	ري رو ور		Raffo Afficial	=	0.51		002	200	1 6	ڪ چ	ı	0.93	ı	1	0.61	1.51	ı	0.52	0.65	0.10	1	1,31	1	0.83	ŧ	1	12.0	3	1 5	777	0,		0.45	99	ı	8 2	5 .0	1 1
X	Weekday		10 V	F/102.8	C 02/2	E/GD 2	7.00.7	200	4700	57.16.3	6718.9	D/36.9	0/36.9	D/40.9	0/529	F/279.2	F/219.4	D/45.0	D/45.8	A0.1	C31.4	F/148.7	F/148.7	C214	C/21.4	L'MIL'A	2,725	240	200	7 0	200	0.00	B/14.9	500	CZS	A9.9	9 5	C/23 B
	day ay		Ratio K	0.51	0.38		0 60	5	1 6	C.30	ı ¦	0.40	ı	1	0.42	0.68	ı	0.45	0.36	0.05	ı	1,19	1	0.61	1	1 8	2 2	i i	1	3 5	27.7	1	44.0	3	ı	E 2	50.0 0.03	1 1
BASELIN	Saturday Midday		LOS/ Delay	V11.7	A/9.8	P/10 7	747.5	245	5.54	No.	Ş.	BV13.7	A/13.7	B/15.7	D35.3	C/27.7	8	C/226	C/20,7	A0.1	B/17.4	F/126.2	F/126.2	B/15.8	BV15.8	100	200		0.000	3 5	75.0	0.00	B/129	B/18.4	B/17.9	A/9.2	7.50	C C C S C
TIONS	day oon		agic Sabi	0.85	0.44	1	07.0	2	1 8	20.0	ij	95. O	ı	-	<u>8</u> 0	1.76	ı	0.34	0.73	0.14	1	203	ı	0.72	1	1 8	S S	50.0	1 0	200	3.5	1 8	0.49	0.0	1 8	0.10	9.0	1 1
2017 EXISTING CONDITIONS (BASELINE)	Weekday Afternoon		LOS/ Delay	D/37.8	A/R G	C/23	180	200	0.000	22.2	577	B/1/.0	BV17.0	C21.B	F/84.3	F/388.2	F/299.9	C24.0	D/35.6	A/0.2	C/Z3.6	F/491,9	F/491.9	B/19.2	B/19.2	F/246.3	252		7.007	ARR	70.0	265.2	6716.6	C723.3	5226	B/122	7 500	C 28 2
EXIST	ng y		Ratio K	0.97	0.48	1	80	3	1 6	ر ال	r	8.0	1	-	0.60	1.37	ť	0.49	0.85	0.09	1	2	ı	0.88	i	1 8	9 9	2	1 5	2		1 5	5.6	3	1 8	8 6		1 1
2017	Weekday		Delay Delay	E/62.1	B/19.4	D/79.8	D/44 1		1 2 2	250	2750	2	3	C34.5	D/524	F/221.5	F/173.6	D/43.4	E/90.7	A/0.1	D#2.7	F/119.1	F/119.1	C/21,8	C/21.B	1,023	2,02,0	7 20	2552	A/1 2	247.0	0.25	0.44	6713.3	B/19.4	27.07	5.45	C/23.1
_			PHYSICAL	EB 1,5		Арр	. I HAM	004		20	- 1	¥ :	APP	Overall		-	APP.	J SM		22	-	E 99	_	17 88	APP.		 	400	2 08	1 1	2 O Y		- F	¥ !	æ.	3 3 3	≥ ;	Overall
			CONTROL	Traffic	Signal										raffic	Signal										1	Cianal	olytici.										
			INTERSECTION	State Route 137	(High Ridge Road)	21 State Route 15	(Merritt Parloway)	Coodhhound Dame	Squinouing regulas						State Route 137	(High Ridge Road)	at State Route 15	(Merritt Parkway)	Northbound	Ramps/Buxton Farm	Road					Pinio Danda 417	State Foure 137	of Coder Doinhte	Boad/Tim of Direc	Boad	noon.							

Notes:

Synchro 9.0 results are used for capacity analysis.
Level of Service determining parameter is called the service measure.
For Signalized Intersections: Level of Service/Average Total delay per vehicle (seconds/hehicle).
ITE publication for Traffic Access and Impact Studies for site development "A Recommended Practice" indicated that overall Level of Service ratings of A to D are normally considered acceptable for signalized intersections (Level C or better are considered desirable). Levels of Service E and F are normally undesirable.

- VIC ratio indicates the amount of congestion for each Lane Group. Any VIC ratio greater than or equal to one indicates that the Lane Group is operating at above capacity.

 R1 = Right Turn to State Route 137 (High Ridge Road).

 R2 = Right Turn to State Route 15 (Merritt Partway) Northbound On-Ramp.

 Physical Units consist of the following:

 1. Lane Group, Approach and Intersection Overall for Traffic Signal Controlled Intersections.

WB = Westbound APP, = Approach SB = Southbound R = Right Tum Frederick P. Clark Associates, Inc. 01631 at the Librar Balley, Bandandure 2017 and et dec 02202011 EB = Eastbound T = Through NB = Northbound L = Left Turn

Table 6
CAPACITY ANALYSIS RESULTS FOR UNSIGNALIZED INTERSECTIONS – MEASURE OF EFFECTIVENESS (MOE) AND IMPACT ASSESSMENT – PEAK HOURS
Proposed New LifeTime Finess Building
High Ridge Park
High Ridge Park
Sammont, Connection

											New York	Statitiona, Confidentical	mean												
			ຂ	17 EXISTI	2017 EXISTING CONDITIONS (BASELINE)	SNOLL	BASELINE		Ŕ	9NO-80	2019 NO-BUILD CONDITIONS (BASE)	SNOLL	(BASE)			2019 BUILD CONDITIONS	D COND	E ONS			PROJE	CT IMPACTS	PROJECT IMPACTS (NO BUILD TO BUILD)	(CIIII)	
			Wee	Weekday	Weekday	day	Saturday	lay.	Weekday	<u>_</u>	Weekday	_	Saturday	Ĺ	Weekday	=	Weekday	ŝ	Saturday						
			Mon	Morning	Апетооп	- LO	Midday	/E	Momin	-	Аветооп	E	Midday		Moming	¥	Afternoon	2	Midday	Weekd	Weekday Moming	Weekdan	Weekday Afternoon	Saturday Midday	Midday
												-	-		\vdash			_	L		Project	_	Project		Project
	CONTROL	PHYSICAL	8	Š	8	Š	8	Š	200	NC L	700	NC L	N N N	V/C LOS/	_	VIC LOS	× ×	8	8	÷	Delay	Change	Delay	Clange	Delay
INTERSECTION	TYPE	UNITS	Delay	_	Detay	Ratio	Delay	_	Delay F	_	\dashv	_	Delay Ra	긕	-	_	\dashv	_	\dashv	SOT ui o	(Seconds)	SOTui	(Seconds)	N LOS	(Seconds)
State Route 137 (High	TWSC	1 BS	A/6.7	0.35	A/2.6	0.17	A1.5	0.13	A77.1 (0.35	A/3.3 0	0.19 A	A1.7 0.	0.15 A7.2	7.2 0.36	36 A3.3	3 0.19	9 A/1.8	0.15	No S	0.2	No No	0.0	왕	0.1
Southbound Left Turn																									
Northbound On-Ramp									_		-				+	_	_		\dashv						
Turn of River Road at	AWSC	151 63	C/15.6	9.54	C22.2	0.70	A/8.7	_	F/206.2	_	-	_	_	=		_	1.00	D B/12	_	_	-56.3	п. П.	19.5	ş	-0.4
Buxton Farm Road/		WB 121	A9.6	0.0	BV10.9	0.0	A8.1	_	_	_	_	_	_	0.02 B/11.5	_	02 By12.9	_			_	6.9	ş	0.1	£	0.0
Assisted Living		NB 1-1	031.5	0.86	0726.3	075	A10.0	0.33	=	1.27 E	_	0.93 BV	B/13.1 0.		_		_	BV128	8 0.45	_	-23.5	E-F	14.6	욷	6 3
Access Drive		25 65	\$	90.0g	C24.6	0.76	A7.5		_	_	7140.1	_	_	0.22 B/12.4	24 0.22	22 F/93.2	1.08		_	_	6.0	ş	46.9	ş	0.4
		Overall	C24.8		C24.3	1	A9.3	1	_		_	-	_	- F/111.4	1.4	- F/172	1	B/12.0		운	41.0	No	-129	욷	9.0
Tum of River Road at	AWSC	5	B/11.4	0.03	A93	00	A/8.1	Н	B/11.9	0.00	H	0.12 A	A/8.4 0.		-		H	\vdash	4 0.02	L	-0.2	Ş.	1.0	2	0.0
Intervale Road/Access		151 BAN	F/90.3		B/11.1	0.31	A9.8			_		-	_	0.44 F/112.0		1.15 E/37.0	.0 0.87	7 BV11.0	-		-25.9	D-E	5.4	욷	0.0
Drive		NB 52	C/15.6	0.46	A/10.0	87	A/8.2	0.12	_			0.41 A	_	_	-	0.49 B/14.5	1.5 0.43	-			17	2	9.0	ş	0.1
		-5 -88	F/53.3		C20.7	27	B/10.3	_	-	_	_	1.07 B	_	0.42 F/6K	F/66.9 1.0	1.05 F/66.7	101	1 8/11.9			9:1	No.	-14.2	£	0.3
		Overall	F/64.1	1	C16.2	1	A/9.7	1	F.91.1	ı	F51.1	 B	B/10.9	- F/79.	_	- F#5.8	-	\dashv	-	_	-120	No	-5.3	묏	0.1
High Ridge Park Road	TWSC	WB L	N/A	NA	N/A	NA	N/A	N/A	N/A	N/A	N/A	N/A	N/A N/A	N/A A0.0	_	0.00 A/0.0	0.00	0 A@0	0.00	WA C	NA	Ϋ́	NA	¥.	N/A
at Proposed Site		NB L	N.	NA NA	W.	N/A	¥	¥	N/A	_	_		_	WA C13.1	_	11 C/16.2				_	¥	¥	S S	ž	¥
Access Drive										-		-	-	-	+	-	-		4						

Synchro 9.0AHCM 2010 results are used for capacity analysis.
 Level of Service determining parameter is called the service measure.

TWSC = Two-Way STOP Control.

AWSC = Iwo-Way STOP Control.
 For TWSC and AWSC Intersections: Level of Service/Average Comtrol delay per vehicle (seconds/vehicle).
 V/C ratio indicates the amount of congestion for each Movement and Lane. Any V/C ratio gneater than or equal to one indicates that the Movement and Lane is operating at above capacity.
 Physical Units consist of the following:

 TWSC Intersections: Critical Lane and Critical Movement.
 AWSC Intersections: Lane and Intersection Overall.

NB = Northbound L = Left Tum

WB = Westbound Ln = Lane SB = Southbound R = Right Turn EB = Eastbound T = Through

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STORAGE/OUGUE ANALYSIS RESULTS FOR SIGNALIZED INTERSECTIONS - PEAK HOURS
Proposed New Life Time Fitness Building
High Ridge Park
Starnford, Connecticut

_	_		_			_			_		_		_							_					
NS	Saturday	Midday	Queue	Length	(Feet)	78	88	1 50	138	202	115	785	Ď	161	0	085	217	114	221	æ	17	18	383	9	582
2019 BUILD CONDITIONS	Weekday	Afternoon	Onene	Length	(Feet)	207	121	155	171	573	293	662	135	448	0	1145	111	147	400	147	ಸ	8	235	\$	215
2015	Weekday	Morning	Onene	Length	(Feet)	337	237	416	102	634	189	616	22	421	0	852	410	98	222	114	89	88	365	<u>6</u>	279
(BASE)	Saturday	Midday	Oueue	Length	(Feet)	08	88	146	128	203	115	774	8	151	0	3	82	115	ន	85	5	SS.	384	5	162
2019 NO-BUILD CONDITIONS (BASE)	Weekday	Afternoon	Quene	Length	(Feet)	217	118	157	181	224	263	9 5	143	515	0	1132	1/2	144	397	1 33	8	25	536	ŧ	325
2019 NO-B	Weekday	Morning	Quene	Length	(Feet)	365	247	428	88	648	189	199	87	252	0	980	2 05	8	771	116	80	25	362	<u>13</u>	268
ASELINE)	Saturday	Midday	Oueue	Length	(Feet)	11/	14	#	136	182	114	961	8	121	0	35	202	103	173	71	o	19	395	17	310
2017 EXISTING CONDITIONS (BASELINE)	Weekday	Afternoon	Queue	Length	(Feet)	821	53	191	170	210	529	286	125	399	0	1099	247	139	Ř	138	8	88	228	16	318
2017 EXISTIP	Weekday	Morning	Oueue	Length	(Feet)	294	225	420	1 02	584	187	98	8	380	0	834	408	-81	727	107	45	61	999	14	283
					PHYSICAL UNITS	3 B3	~	WB T	⊢ 8	SB TR	1 83	Ħ	WB L	~	22		SB L1	1 69	Ħ	We L	¥	NB L	¥	28 L	똤
				STORAGE	LINK LENGTH	1,900	320	165	625	160	1,330	92	2 5	230	92	380	625	175	460	901	300	23	92	3 5	002
					CONTROL TYPE	Traffic Signal					Traffic Signal	1						Traffic Signal							
					INTERSECTION	State Route 137 (High Ridge	Road) at State Route 15	(Merritt Parkway) Southbound	Ramps		State Route 137 (High Ridge	Road) at State Route 15	(Mentit Parkway) Northbound	Ramps/ Buxton Farm Road				State Route 137 (High Ridge	Road) at Cedar Heights	Road/Tum of River Road					

Notes:

Synchro 9.0 Macroscopic model is used for storage/queue analysis.

The Queue Length rows show the 95th percentile maximum queue length in feet.

The Queue Length is for each lane. The total queue length is divided by the number of lanes and the lane utilization factor.

The SSP percentile queue is the maximum back of the queue with the 95th percentile traffic volumes.

Bolded SSP percentile queue exceeds the strange available.

RI = Right I um to State Route 137 (High Rodge Road).

R2 = Right I um to State Route 137 (High Rodge Road).

Physical Units consist of the following:

1. Lane Group for Traffic Signal Controlled Intersections.

NB = Northbound L = Left Turn

WB = Westbound SB = Southbound R = Right Turn EB = Eastbound T = Through

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STORAGE/QUEUE ANALYSIS RESULTS FOR UNSIGNALIZED INTERSECTIONS – PEAK HOURS
Proposed New Lifetime Fitness Building
High Ridge Park
Stamford, Connecticut Table 8

				2017 EXISTIN	2017 EXISTING CONDITIONS (BASELINE)	3ASELINE)	2019 NO-B	2019 NO-BUILD CONDITIONS (BASE)	(BASE)	2019	2019 BUILD CONDITIONS	NS
				Weekday	Weekday	Saturday	Weekday	Weekday	Saturday	Weekday	Weekday	Saturday
				Morning	Afternoon	Midday	Morning	Afternoon	Midday	Moming	Afternoon	Midday
		STORAGE		Onene	Quene	Oueue	Onene	Ovene	Quene	Queue	Queue	Queue
		LINK LENGTH		Length	Length	Length	Length	Length	Length	Length	Length	Length
INTERSECTION	CONTROL TYPE	(Vehicle)	PHYSICAL UNITS	(Vehicle)	(Vehicle)	(Vehicle)	(Vehicle)	(Vehicle)	(Vehide)	(Vehicle)	(Vehicle)	(Vehicle)
State Route 137 (High	TWSC	19.4	7 BS	1.6	1.1	0.5	1.6	1.1	0.5	4.0	2.1	1.8
Ridge Road) at Southbound												
Left Turn Northbound On-												
Ramp												
Turn of River Road at	AWSC	21.0	5 8	3.2	5.7	1.2	36.9	9.7	3.1	29.0	13.3	2.8
Buxton Farm Road/		0.9	WB L-1	0.0	0.0	0.1	0.0	0:0	0.1	0.0	0.0	0.1
Assisted Living Access		34.2		10.0	6.6	4.1	19.1	9.8	2.5	16.4	12.2	2.4
Drive		24.0	SB Ln1	0.3	6.9	0.1	0.7	26.4	0.8	0.7	18.6	Ξ
Turn of River Road at	AWSC	3.0	EB Ln1	0.1	0.0	0:0	0.1	0.1	0.0	0.1	0.1	0.0
Intervale Road/Access Drive		47.2	WB Ln1	20:0	1.3	1.6	26.9	8.6	2.2	23.0	10.1	21
		19.0	NB 52	2.2	1.2	0.4	2.6	6.1	0.5	2.4	2.1	0.5
		13.6	SB Ln1	12.0	6.6	1.4	13.4	17.8	5.0	13.9	15.2	2.2
High Ridge Park Road at	TWSC	10.0	WB L	N/A	N/A	N/A	NA	NA	N/A	0.0	0.0	0.0
proposed Site Access Drive		5.0	15 BR	NA NA	¥8	N/A	N/A	N/A	¥N.	0.4	23	1.0

Notes:

Synchro 9.0 Macroscopic model/HCM 2010 results are used for storage/queue analysis.

The Queue Length rows show the 95% percentile maximum queue length in vehicles.

The Queue Length is for each lane. The total queue length in vehicles.

The Queue Length is for each lane. The total queue length is divided by the number of lanes and the lane utilization factor.

The 95% percentile queue is the maximum back of the queue with the 95% percentile traffic volumes.

Bolded 95% percentile queue exceeds the storage available.

TWSC = Two-Way STOP Control.

N/A = Not Available.

Physical Units consist of the following:

1. TWSC Intersections: Critical Lane and Critical Movement.

T = Through

Lı = Lane

Frederick P. Clark Associates, Inc. G-653.400 New LifeTane Figness Busking, Stamford.Lune 2017wordtif 17-008 sta. doc 614.177

For the purposes of completing the Study, the proposed development is expected to be completed and fully occupied by the end of 2019.

This Traffic Study addresses traffic conditions for the 2017 existing, the 2019 future no-build and build conditions during the weekday morning, weekday afternoon and Saturday midday peak hours of the adjacent street system. The 2017 baseline traffic volumes were extracted from manual turning movement counts conducted for all but one of the Study Area intersections in January and March of 2017 during the weekday morning, weekday afternoon and Saturday midday peak periods. Traffic volumes for the intersection of High Ridge Road at Cedar Heights Road/Turn of River Road were derived from another Traffic Study prepared by this office.

Future no-build traffic volumes, without the proposed development, assumed that the currently vacant Building 3 was re-occupied with medical uses. A 0.6 percent annual growth rate was employed to the existing traffic volumes to the design year of 2019 as required by ConnDOT. No other developments were identified by either the City of Stamford Planning Department or the ConnDOT Planning Division for inclusion in this analysis. The annual growth rate is consistent with the City of Stamford and ConnDOT rates.

Traffic generation for both the vacant Building 3 re-occupancy with medical uses and the proposed LifeTime Fitness building are based on trip generation rates provided by the Institute of Transportation Engineers (ITE). It is anticipated that the removal of re-occupied Building 3 with medical and the 6,128 square-feet of office space (to be removed) were to remain, it is anticipated that it would generate a total of 210, 309 and 308 vehicle trip ends during the weekday morning, weekday afternoon and Saturday midday peak hours, respectively.

It is anticipated that the proposed Fitness Center will generate a total of 161, 402 and 317 total trip ends, of which 56, 141 and 0 are internal trips and 105, 261 and 317 are external vehicle trip ends during the weekday morning, weekday afternoon and Saturday midday peak hours, respectively. The

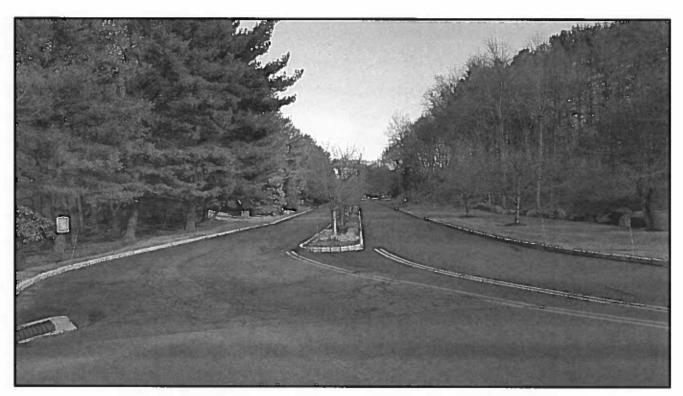
net difference between the re-occupancy of Building 3 with medical uses and the proposed development is 105 and 48 fewer vehicle trip ends during the weekday morning and weekday afternoon peak hours and an increase of 9 vehicle trip ends during the Saturday midday peak hours, respectively. Thus, the proposed development will have a reduced traffic impact on area roadways during weekday peak hours compared to the no build condition and only generate 9 additional trips during the Saturday midday peak hour.

Based on a review of current traffic patterns at Study Area intersections, anticipated travel routes to the site and guidance from ConnDOT, a distribution plan was developed. As described above, the site access drives will be to the internal High Ridge Park driveway. It is assumed that all traffic will use the northerly internal access drive, to be conservative. It was determined that 20 percent of the site traffic will arrive from and depart to the east and west (each way), respectively, on State Route 15, 25 percent arrive from and depart to the east on Intervale Road, 15 percent will arrive from and depart to the north on High Ridge Road, 13 percent will arrive from and depart to the south on High Ridge Road and 7 percent will arrive from and depart to the west on Cedar Heights Road.

Future 2019 build traffic volumes were developed based on adding the net increase in site traffic generation to the 2019 no-build traffic volumes, as previously described.

SYNCHRO 9 macroscopic capacity analysis were conducted for the 2017 existing, 2019 nobuild and build conditions to identify incremental traffic impacts and needs that the proposed development will generate during peak hours.

Results of the capacity analysis and the storage/queue analysis indicate that the Study Area intersections will continue to operate with traffic congestion during the Study Area peak hours. However, this condition is not exacerbated by the proposed development. In fact, in some instances, conditions will improve, and the construction of the proposed development will result in a net decrease in traffic impact during peak hours when compared to the permitted office use on the Campus.



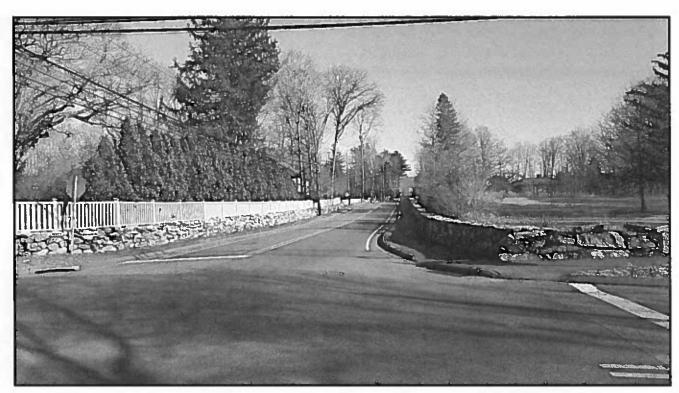
High Ridge Park at Turn of River Road Looking East



Assisted Living Access Drive at Turn of River Road Looking East



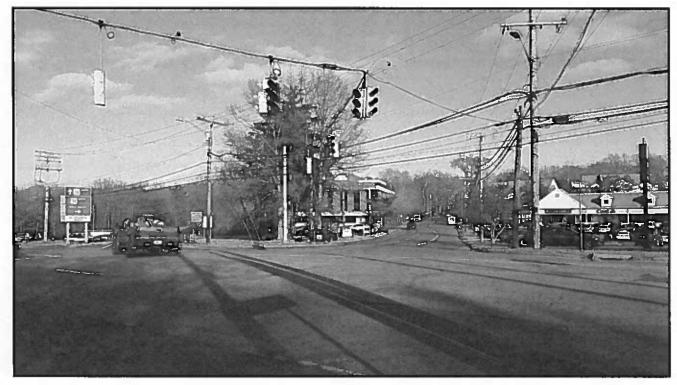
Turn of River Rd. at Buxton Farm Rd. / Assisted Living Access Drive Looking South



Intervale Road at Turn of River Road Looking East



Turn of River Road at Intervale Road / Access Drive Looking South



Buxton Farm Road at High Ridge Road Looking East



High Ridge Rd. at SR 15 Northbound Ramps / Buxton Farm Rd. Looking South



On-Ramp to State Route 15 Southbound at High Ridge Road Looking East



High Ridge Road at State Route 15 Southbound Ramps Looking South